Steffany Ngu

Subject: EBP traffic modelling review with Mott MacDonald

Location: Microsoft Teams Meeting

Start: Tue 11/02/2025 4:00 PM End: Tue 11/02/2025 5:00 PM

Show Time As: Tentative

Recurrence: (none)

Organizer: John Harrison

Vu; Howa Janice Buteux-wheeler; Colin Muir; Umesh Murdeshwar; Eric Wu; Howard Chan; Jo **Required Attendees:**

Kaya; Andrew Ha

Optional Attendees:

Microsoft Teams meeting Need help?

Join: S 74 out of scope

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For organizers: Meeting options | Reset dial-in PIN Released unde

- TfNSW review of modelling noted in summary that (refer details in the email below):
 2039 "Do Minimum" (without project) scenario has not reported

 Calibration (SIDRA), coding and signal coordination issues (SIDRA and VISSM) identified
- Reporting does not provide an accurate assessment of the project's benefits or impacts, particularly regarding delay, queuing, and travel time. Indications are that Mott Macdonald results inaccurately indicated worse results. The REF may therefore be in error.

- TfNSW modelling team will provide their updated data to be passed onto Mott MacDonald
- 2. Mot MacCandia to review availability and timing to update their model and the REF findings.
 3. Follow up meeting with Mott MacDonald and project team to confirm the actions to progress the matter Colin are you able to facilitate please?

and thanks	s all for the	meeting today v	whereby I note we discussed the following:-						
			mmary that (refer details in the email below):-						
 2039 "Do Minimum" (without project) scenario has not reported Calibration (SIDRA), coding and signal coordination issues (SIDRA and VISSM) identified 									
Report	ing does no	ot provide an acc	ccurate assessment of the project's benefits or impacts, particularly regarding delay, queuing, and travel time. Indications are that Mott Macdo	onald results inaccurately indicated worse results. The REF	().				
may therefore be in error.									
It was resolved that:-									
			de their updated data to be passed onto Mott MacDonald		XV				
			bility and timing to update their model and the REF findings. Donald and project team to confirm the actions to progress the matter – Colin – are you able to facilitate please?						
					ct. 200°.				
l suggest	the above	actions to be ur	undertaken tomorrow 12/2/25 to allow the necessary modelling and REF updates to progress and resolved ASAP after that.	< Y					
ıks very n	nuch Howa	rd and Eric for y	your prompt assistance to understand the issues.						
- for	tracking, p	olease refer to e:	extracts from the TfNSW EBP Technical comments Register (TCR TM030 & TM032), below as these indirectly refer to these issues.	63,					
. 30	19/4/24	Traffic Impact	10/2 - JH - noted advice - essential the modelling is accurate and record and ideally mitigate any impacts to the surrounding road network. Accordingly, it has been agreed that the	20/12-JH-noted ADC log includes an entry (Item 77) that refers to this issue and transfers action to TfNSW to consider possible mitigations in future	1				
		Assessment	project team will request VISSIM modelling to be done to accurately reflect the benefits and impacts of the project. And subsequently, the REF is to include mention of all of this and acknowledge any impacts to other intersections in the area, despite any further work not being included in the current scope of the project. 25/1-II-H-awaiton on further VISSIM model review by TINSIWM Internal and SIDRAI (required.	and transfers action to TINSW to consider possible mitigations in future projects located on Carlingford Road and Ray Street. This is out of project scope. Note that comments on REF relate to this. Refer to these responses					
			25/11/2042-Ti- It is critical that it is clearly noted within key project documentation that traffic impacts are expected as a result of this project, however, those impacts are beyond project scope, etc. Unless NSS have seen that this has been included, we can't close this comment.	(such responses will refer to TINSW for other actions as may be appropriate).					
			15/9-JH-will await updated ADC log 22/7-JH-will await updated ADC log and Traffic Impact Assessment Report	30/08/24 - EW - Traffic report issued, ADC to be submitted along with 100%					
			13/7/24-JH-designer to update ADC log accordingly - re project assumption / constraints 31/5-TI - Agree with this approach. NSS understand the limitations, however, we need to tag this concern and any possible miligations should be considered. 37/5-TI - It - It was a thing to provide additional in continuous professional programment in continuous program	resubmission 18/07/24 - EW - Added ADC77 12/07/24 - Consideration of possible miligations are not part of our scope. The issues fall outside the project boundaries.					
			27/5-JH - to TI - suggest designer provide statement in requirement verification (RVTM) and ADC log that project satisfies Business Requirement and miligated "downstream" impacts where possible within constraints of the project? 98/05/24 - TI hat is understood. however, those intersections are already heavily concested. Due to the town centre insture of the area and upcoming development, any further	scope. The susues fall outside the project boundaries. 24/05/24 - The capacity improvements at Blaxland Road and Epping Road have moved the westbound bottleneck west, to Carlingford Road and					
			impact to these intersections would not only be detrimental for efficiency, but also add to safety concerns of all road users. Impacts should be aimed to be minimised as much as possible.	Ray Street. Additionally, the STFM model forecasts increased traffic growth in future years, particularly in the contra-peak directions. This increased traffic demand at intersections without upgrades, results in worsening					
			1944-Ti-Having a look at all the future year modeling results comparisons, it seems that there is expected to be a negative impact to the efficiency at certain intersection. For example calls approaches at the intersection of CarlingfordRay Rd intersection as well as Epping Road and Essex Street. From a logical assessment, the proposed works should not have a negative impact to these intersections and we question what is causing these results?	traffic demand at intersections without upgrades, results in worsening intersection performance.					
32	19/4/24	Traffic Impact Assessment	10/2 - JH - noted advice - essential the modelling is accurate and record and ideally mitigate any impacts to the surrounding road network. Accordingly, it has been agreed that the project team will request VISSIM modelling to be done to accurately reflect the benefits and impacts of the project. And subsequently, the REF is to include mention of all of this and	20/12-JH- Further review required by Tanmila and Peta if we want MM to adjust anything noting that there is no further modelling in project scope in					
			acknowledge any impacts to other intersections in the area, despite any further work not being included in the current scope of the project. 25/1-JH-lawaking on further VISSM model review by TINSWW internal and SIDRA if required. 117/1-JH- TI please review	this regard. 12/07/24 The reviewer states this is not a hold point. The 2039 Do Minimum is an artificial and worthless comparison. Delay would never					
			28/5/24 -TI - I don't think my comment has been understood. Happy to discuss over phone if needed. This isn't a holding point for the project, but consistency in the report is preferred.	continue to build to those ridiculous levels. Drivers would change their behaviour.					
			8/05/24 - Til- This is understood, and the comment is not regarding the result of the 2039 scenario, but moreso in relation to the comparison that is made. Comparisons should be made between Do-Nothing vs Project scenarios in the same years to demonstrate the impacts/benefits of the proposal in each of those years. Comparing 2023 to 2039 doesn't	24/05/24 — performance levels begin to drop again. Beyond a certain performance threshold, demand stabilises and travellers begin to change the					
			provide this. 1914-T1-A conclusion has been derived that the 2039 Project scenario will operate much worse than the 2029. However, this is not a reasonable comparison as the road environment and traffic volume will change over the 10 years. Ideally a comparison of the 2039 Do nothing scenario with a 2039 Project scenario should be ognisidered.	time of departure (peak spreading), they change their route choice, or they change their mode.					
d regards			1904-TiAR conclusion has been derived that the 2039 Project scoration will operate much worse than the 2009. However, this is not a reasonable congagnition as at the road environment and traffic volume with change over the 10 years. Ideally a comparison of the 2039 Do nothing scenario with a 2039 Project scenario should be climidated.		•				
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arvest St,	Macquarie F	Park NSW 2113							
	Transport								
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	, 11 Februar	ry 2025 10:00 AM							
			.cwheeler@transport.nsw.gov.au>; Colin Muir <colin.muir@transport.nsw.gov.au>; Umesh Murdeshwar <umesh.murdeshwar@transport.nsw.gov.au> Jo Kaya <joanna.jarraldkaya@transport.nsw.gov.au>; Andrew Ha <andrew.ha@transport.nsw.gov.au>; @mott</andrew.ha@transport.nsw.gov.au></joanna.jarraldkaya@transport.nsw.gov.au></umesh.murdeshwar@transport.nsw.gov.au></colin.muir@transport.nsw.gov.au>	>; Eric Wu <eric.wu2@transport.nsw.gov.au>; Howard Chan ttmac.com></eric.wu2@transport.nsw.gov.au>					
			@mottmac.com>	maccome					
bject: KE: E	BP traffic mo	adelling review w	with Mott MacDonald						
	- latest	t review by TfNS	SW traffic modelling team is as follows for further discussion at the meeting today please						
ease also p	pass on / inv	vite a	as necessary)						



SIDRAPM - Sensitivity (TfNSV	r):															
			2023 PM	Base			2039 PM DM				2039 PM Bridge Upgrade					
	Total Arrival Flow (veh/hr)	DOS	Average Delay (sec)	LOS	95% back of Q (m)	Survey Q (m)	Total Arrival Flow (veh/hr)	DOS	Average Delay (sec)	LOS	95% back of Q (m)	Total Arrival Flow (veh/hr)	DOS	Average Delay (sec)	LOS	95% back of Q (m)
Carlingford / Ray/Rawson	2780	0.87	25	В	230	210	3110	0.96	41	С	365	3169	0.97	44	D	385
Carlingford / Beecroft	4671	1.03	36	С	310	210	5005	1.05	63	Е	496	5145	1.09	77		534
Epping / Blaxland	5078	1.01	33	С	285	308	5468	0.98	31	С	321	5421	0.89	26	В	241
Epping / Essex	3306	0.95	42	С	221	252	3531	1.04	56	D	331	3507	1.03	57	Ε	287
Epping / High	4797	1.04	60	E	92	49	5271	1.54	>500		184	5231	0.72	6	Α	35
Rawson / Bridge	1097	0.71	24	В	30	56	1340	1.14	>100	F	183	1347	1.02	76	F	103

Caringiola / Ray/Ray			23		230	210	5110	0.50	71		303	3109	0.57	44		303
Carlingford / Beecrof				С	310	210	5005	1.05	63		496	5145	1.09	77	F	534
Epping / Blaxland	507		_	С	285	308	5468		31	C D	321	5421	0.89	26	В	241
Epping / Essex	330			С	221 92	202	3531		56 >500	0	331	3507	1.03	57		287
Epping / High	479 109			E B	92 30	49	5271 1340		>500 >100	F	184 183	5231 1347	1.02	6 76	A	35 103
Rawson / Bridge	109	0.71	24	В	30	56	1340	1.14	>100	- 1	183	134/	1.02	76	-	103
SIDRA PM - Original (Mott McDoneld\															
SIDION PIN - Original (mou incoondid):		2023 PW	Base				2039	PM DM				2039 PI	M Bridge Upg	grade	
	Total A	rrival	Average	T	orar ha		T-1-134-1		verage		orev by t	Total Arrival		Average		oras bu
	Flov			LOS	95% back of Q (m)	Survey Q (m)	Total Veh (veh/hr)	DOS	Delay	LOS	95% back of Q (m)	Flow	DOS	Delay	LOS	95% back of Q (m)
Carlingford / Ray/Ray	(veh/ wson 283		(sec) 67	-	137	210	,		(sec)			(veh/hr) 3156	2.72	(sec) 161	F	378
Carlingford / Ray/Ray Carlingford / Beecrof		_	_	C	137	210	1					3156 4936	1.26	161		378
Epping / Blaxland	t 460			C	125	210 308	ł					4936 5323	0.81	68 25	В	128
Epping / Essex	327		_	C	185	308 252	1	Did not	modelled	d		5323 3449	0.81	25 46	D	130
	327 472				135	252	1					3449 5115	0.89	46 5	U	130 41
Epping / High Rawson / Bridge	109			D	12	49 56	1					1288	1.282	97	A .	25
Rawson / Bridge	109	7 1.10	40	υ	12	56						1288	1.282	97		25
enh Ir 2. <mark>2039 "Do</mark>	ed time signal lance interse nefficient sign	ls were im ction perfo nal coordii cenario:	ormance in nation was	the proje	ect case. ed, resultin	ing in incr	reased dela	ays and hig	h level	oflaten	it daman	d while oth	er locatie	on / direc	etion have	a enara (
Ţ	he network re	esults fron	Mott McD	onald's	original 2	2039 PM p	project mod	del were sig	gnifica	ntly wor	se comp	ared to our	new cre	ated 203	9 "Do Mi	inimum"
C	apacity but p	oorer in o	reratt netw	огк репо	ormance.	1										
3. Coding Iss	sues:															
	eral coding is	sues, suc	h as incom	ect confl	lict marke	ers, were	found prin	narily in the	projec	ct case.	creating	unnecessa	rv delav	s or block	kages.	
Below are the VI										the sign	al coordi	nation issu	e and ad	finsted se	everal ne	twork/n
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	20000	2039 AM DM	2039	AM Project												
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Avg Delay (s)	112	129	121	11	14											
Avg Speed (km/h)	26	24	25	20	6											
VKT (km)	9651	10156	10115	103	370											
VHT (hr)	369	425	408	40												
Total Veh	6119	6534	6509	664	40											1
Latent Demand	5	289	310	30	10											- 1
ļ																
Network Stats	2023 Base PM	2039 PM DM		PM Project											-1	
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Avg Delay (s)	98	216	240	19												1
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Latent Demand	0	90	419	16	96									10		
				1				2039 AM Pro			- 1			. "		
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1: Beecroft Rd NB 2: Beecroft Rd NB		470	55.3	486	54.7		000		463	88.9						
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2 4: Beecroft Rd SB		1280	63.8	1210					209	68.0	$\overline{}$	*				
5: Beecroft Rd SB		346	154.0	365	158.				369	73.5	7					
6: Beecroft Rd SB	100,00								556	26.7						
	-R2 3	553	25.2	557	_	2	562 2	25.7								
7: Carlingford Rd/		_	25.2 214.4	557 1075	25.2						-					
7: Carlingford Rd/	Epping EB - R3_1	553 1071 999	25.2 214.4 54.0	557 1075 1023	25.2 218.	3.5 1	1071 22	22.1 1	057	89.5 60.5						
8: Carlingford Rd/	Epping EB - R3_1 Epping EB - R3_2	1071	214.4 54.0	1075 1023	25.2 218. 56.6	3.5 1 .6 1	1071 22 1019 5	22.1 1	057	89.5 60.5						
8: Cartingford Rd/	Epping EB - R3_1 Epping EB - R3_2 Epping EB - R3_4	1071 999 1776	214.4	1075	25.2 218. 56.6	3.5 1 .6 1 .8 1	1071 22 1019 5 1696 4	22.1 1 38.1 1 14.4 1	057 003 718	89.5						
8: Carlingford Rd/	Epping EB - R3_1 Epping EB - R3_2 Epping EB - R3_4 1/Epping WB - R4_1	1071 999 1776 1 667	214.4 54.0 47.8	1075 1023 1698 746	25.2 218. 56.6 45.8	3.5 1 .6 1 .8 1	1071 22 1019 5 1696 4 754 8	22.1 1 38.1 1 44.4 1 85.8	057	89.5 60.5 70.5						
8: Carlingford Rd/ 9: Carlingford Rd/ 10: Carlingford Rd	Epping EB - R3_1 Epping EB - R3_2 Epping EB - R3_4 I/Epping WB - R4_1 I/Epping WB - R4_2	1071 999 1776 1 667 2 607	214.4 54.0 47.8 92.0	1075 1023 1698	25.2 218. 56.6 45.8 92.2	3.5 1 .6 1 .8 1 .2 3	1071 22 1019 5 1696 4 754 8 717 6	22.1 1 38.1 1 14.4 1 35.8 3 30.2	057 003 718	89.5 60.5 70.5 98.0						

Network Stats	2023 Base AM	2039 AM DM	2039 AM Project			
Network Stats	2023 Base AM	2039 AM DM	Mott McDonald	TfNSW Sensitivity		
Avg Delay (s)	112	129	121	114		
Avg Speed (km/h)	26	24	25	26		
VKT (km)	9651	10156	10115	10370		
VHT (hr)	369	425	408	406		
Total Veh	6119	6534	6509	6640		
Latent Demand	5	289	310	30		

Network Stats	2023 Base PM	2039 PM DM	2039 PM Project			
Network stats	2023 Base FM	2030 FFI DIN	Mott McDonald	TfNSW Sensitivity		
Avg Delay (s)	98	216	240	197		
Avg Speed (km/h)	28	16	15	17		
VKT (km)	9563	10311	10015	10273		
VHT (hr)	345	639	666	593		
Total Veh	6252	6969	6752	6900		
Latent Demand	0	90	419	166		

			123 Base AM	١.	039 PM AM	2039 AM Project				
	Travel Time	20	IZ3 Base AM	-	U39 PM AM	Mo	ett McDonald	TfNSW Sensitivity		
		Total Veh	Avg Travel Time (S)	Total Veh	Avg Travel Time (S)	Total Veh	Avg Travel Time (S)	Total Veh	Avg Travel Time (S)	
Т	1: Beecroft Rd NB - R1_1	610	44.6	628	44.7	630	44.2	639	49.8	
	2: Beecroft Rd NB - R1_2	470	55.3	486	54.7	486	55.6	463	88.9	
101	3: Beecroft Rd NB - R1_3	928	39.7	973	39.7	962	39.6	1004	39.5	
Route	4: Beecroft Rd SB - R2_1	1280	63.8	1210	65.2	1211	65.3	1209	68.0	
	5: Beecroft Rd SB - R2_2	346	154.0	365	158.6	359	69.3	369	73.5	
	6: Beecroft Rd SB - R2_3	553	25.2	557	25.2	562	25.7	556	25.7	
Т	7: Cartingford Rd/Epping EB - R3_1	1071	214.4	1075	218.5	1071	222.1	1057	89.5	
	8: Cartingford Rd/Epping EB - R3_2	999	54.0	1023	56.6	1019	58.1	1003	60.5	
ute 2	9: Carlingford Rd/Epping EB - R3_4	1776	47.8	1698	45.8	1696	44.4	1718	70.5	
Rou	10: Cartingford Rd/Epping WB - R4_1	667	92.0	746	92.2	754	85.8	726	98.0	
	11: Cartingford Rd/Epping WB - R4_2	607	48.9	729	52.8	717	60.2	780	90.2	
	12: Carlingford Rd/Epping WB - R4 3	687	45.8	808	48.0	797	47.4	866	43.9	

		20	123 Base PM	,	039 PM DM	2039 PM Project				
	Travel Time	20	23 base FM	_	030 FFI DFI	Mo	tt McDonald	TfNSW Sensitivity		
		Total Veh	Avg Travel Time (S)	Total Veh	Avg Travel Time (S)	Total Veh	Avg Travel Time (S)	Total Veh	Avg Travel Time (S)	
	1: Beecroft Rd NB - R1_1	832	71.3	889	70.8	804	366.9	786	366.7	
	2: Beecroft Rd NB - R1_2	665	57.4	727	65.5	663	69.3	642	79.6	
ute 1	3: Beecroft Rd NB - R1_3	1262	39.5	1427	39.5	1395	39.4	1378	39.4	
Pon Bon	4: Beecroft Rd SB - R2_1	890	53.9	914	61.5	920	61.2	914	60.0	
	5: Beecroft Rd SB - R2_2	180	420.3	237	110.3	232	77.3	238	69.3	
	6: Beecroft Rd SB - R2_3	267	25.1	330	25.3	325	25.9	330	25.8	
	7: Carlingford Rd/Epping EB - R3_1	870	90.1	1009	299.0	870	427.3	1036	332.1	
	8: Carlingford Rd/Epping EB - R3_2	584	40.5	591	42.7	521	39.8	608	43.1	
16.2	9: Carlingford Rd/Epping EB - R3_4	1035	41.8	992	28.0	928	39.5	1007	27.4	
Rou	10: Cartingford Rd/Epping WB - R4_1	1187	80.3	1136	211.2	1260	72.8	1250	55.4	
	11: Cartingford Rd/Epping WB - R4_2	801	66,9	743	66.5	788	93.7	783	91.3	
	12: Cartingford Rd/Epping WB - R4_3	860	46.0	805	45.1	837	47.4	840	44.6	





Base Year Model Technical Note

Epping Bridge Upgrade

Project: Epping Bridge Replacement

Our reference: 703100907 Your reference: EBP150523-MOTTM-

EPG-TP-RPT-000001

Rev C OFFICIAL

Prepared by: Date: 25 July 2024

Approved by: Checked by:

Subject: Base Year Model Calibration and Validation Report

1 Introduction

1.1 Background

This draft technical note documents the development, calibration, and validation of the 2023 Base Case VISSIM and SIDRA models for the Epping Bridge project.

We seek TfNSW review, comment and/or agreement to their fitness for purpose, prior to commencement of the end state scenario modelling and construction staging work for the Concept Design phase.

Traffic modelling is required to demonstrate the potential impacts of the project on the transport network, during construction and at end state, for the Review of Environmental Factors (REF).

This Technical Note has been structured in line with TTD2017/001.

1.2 Project Objectives

The proposed project will replace and widen the existing Epping Bridge across the combined T9 and Metro rail corridor. It will include the following upgrades to Epping Road and Beecroft Road:

- an additional eastbound right turn lane from Epping Bridge into Blaxland Road
- an additional westbound lane across the bridge deck
- widening of the traffic lanes on the bridge deck, continuing around the sharp curve in the Beecroft Road alignment, immediately west of the bridge
- adjustments to the Beecroft Road access with Bridge Street and High Street
- improved active transport facilities along the southern footpath of the bridge
- relocation of existing Beecroft Road bus stops north of the pedestrian overbridge

This document is issued for the party which commissioned it and for specific purposes connected with the above-captioned project only. It should not be relied upon by any other party or used for any other purpose.

We accept no responsibility for the consequences of this document being relied upon by any other party, or being used for any other purpose, or containing any error or omission which is due to an error or omission in data supplied to us by other parties.

This document contains confidential information and proprietary intellectual property. It should not be shown to other parties without consent from us and from the party which commissioned it.

1.3 Scope of Work

Mott MacDonald has been engaged to develop a Preliminary Design Review (Concept Design) and develop the Review of Environmental Factors (REF) which will be used to seek Planning Approval for the project. The Concept Design would then be used as the reference design for the next phase of procurement for the Detailed Design and Construction of the works.

The concept design will be built upon the work completed in 2021 by Future Rail for the Definition Design of the Epping Bridge Project. The Future Rail Definition Design assessed both a widening and replacement option which was used to prepare the Final Business Case, which is now complete.

The purpose of the VISSIM and SIDRA modelling is to:

- Provide traffic analysis for the Review of Environmental Factors (REF) of the Epping Bridge Replacement proposal. The modelling will need to compare traffic performance With and Without Project at opening year 2028 and 10-years after opening.
- Assess the travel delay impacts, if any, of key traffic staging arrangements during construction of the project, to inform the Road User Delay Management Strategy.

Whilst there may be minor design optimisation undertaken during the Concept Design phase, the design layouts were specified in the Concept Design brief. Optioneering and/or comparative assessments of alternate designs are not included in the Concept Design scope.

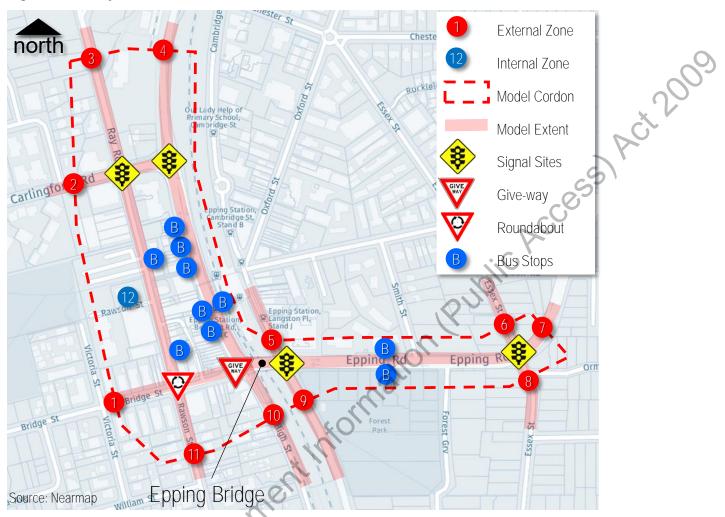
1.4 **Study Area**

The proposed extent of the VISSIM & SIDRA models is presented in Figure 1-1 The 12x12 cordon zones, .1) Government that were adopted for the previous VISSIM modelling during the Reference Design phase, are retained. The zone descriptions include:

- 7. Epping Road (east)
- 8. Essex Street (north)
- Rawson Street (south)
- 10. Blaxland Road (south)
- 11. High Street (south)
- 12. Rawson Street Carpark

Mott MacDonald Page 2 of 69

Figure 1-1 Study Area



1.5 Report Outline

This report has been structured in accordance with TTD 2017/001 and includes:

Section 1 - Introduction

Section 2 – Existing Conditions

Section 3 – Model Development

Section 4 – Model Calibration and Validation

Section 5 - Summary and Conclusion

2 **Existing Conditions**

2.1 **Traffic Surveys**

Classified Intersection Traffic Counts were obtained for following peak periods on Thursday 21/9/2023.
AM Peak Period (06:00-10:00)
PM Peak Period (15:00-19:00)
Traffic data was collected at the Communication. The following data has been collected to undertake the development and calibration of the AM and PM peak

- - Epping Road and Essex Street
 - Epping Road, Langston Road and Blaxland Road
 - Epping Road, High Street and Bridge Street
 - Carlingford Road and Beecroft Road
 - Carlingford Road, Rawson Street and Ray Road
 - Bridge Street and Rawson Street

Based on traffic survey data, the following peak hours were identified in AM peak and PM peak:

- AM Peak Hour (07:30-08:30)
- PM Peak Hour (17:00-18:00)

2.1.2 **SCATS Information**

SCATS Signal operation data for 21/09/2024 was provided by TfNSW for the following signalised intersections:

- Carlingford Road / Ray Road/ Rawson Street (VV1015)
- Carlingford Road / Beecroft Road (VV0706)
- Epping Road / Langston Place/ Blaxland Road (VV0216)
- Epping Road / Essex Road (VV1338)

This SCATs information was analysed for the peak hours to understand the signal phasing times and cycle times. The signal phases given in the SCATS were coded in VISSIM including the alternate phases. The alternate phases were coded as a variable phase and some of these alternative phases were skipped based on the demand. The phases with green time have been retained in the modelling. It was observed that all signalised intersections in the study area are running with a cycle time of ~130 seconds. The user-given cycle time of approximately 130 seconds was adopted for all signalised intersections in the base case scenario.

Queue Length Survey

Intersection Queue Length Surveys at the same survey sites, on Thursday 21/9/2023, are presented in **Appendix B.** This queue length data was used for model validation.

A review of the queue length data revealed the following:

- IC01: Carlingford Road / Ray Road/ Rawson Street.
 - During the AM Peak, a substantial queue of 42 vehicles was observed on the west approach.
- IC02: Carlingford Road / Beecroft Road:

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- In both the AM and PM peaks, a maximum of 30 vehicles were observed queuing on the South approach through lanes.
- IC03: Epping Road / Blaxland Road:
 - During both the AM and PM Peaks, there were 30-35 vehicles queuing to turn right from Beecroft Road's west approach onto Blaxland Road.
 - In the PM peak, a maximum of 40 vehicles were observed queuing to turn left from Blaxland Road's south approach into Beecroft Road.
- IC04: Epping Road / Essex Road:
 - In both the AM and PM Peaks, 13-15 vehicles were observed queuing on the North approach attempting to turn right into Epping Road.

2.1.4 Travel Time Survey

Travel time data was collected on 21/09/2023 during the 4-hour AM and PM peak periods using the floating car method. The Base Case models are validated using this travel time data. The routes for the travel time assessments were:

- Route 1: Beecroft Road and Blaxland Road (Northbound and Southbound)
- Route 2: Carlingford Road, Beecroft Road and Epping Road (Eastbound and Westbound)

In addition to the primary travel time surveys above, HERE travel time data was sourced for the same road segments during the same periods. A summary of the travel time data is presented in **Table 2.1** and travel time graphs are presented in **Appendix C**.

Table 2.1: Travel Time Data

No	Route -	Section	on	AM Peak (07:30 - 08:30)	PM Peak (1	7:00 - 18:00)
NO	Route –	From	То	Survey	HERE	Survey	HERE
		Maida Rd	Epping Rd	0:00:29	0:00:43	0:03:06	0:00:55
1	Beecroft Rd NB	Epping Rd	Carlingford Rd	0:01:26	0:01:08	0:01:23	0:01:11
		Carlingford Rd	Kandy Ave	0:00:42	0:00:54	0:00:40	0:00:29
	Beecroft	Kandy Ave	Carlingford Rd	0:01:04	0:00:52	0:01:18	0:01:35
2	Rd SB	Carlingford Rd	Blaxland Rd	0:02:54	0:01:34	0:06:09	0:01:35
		Blaxland Rd	Maida Rd	0:00:25	0:00:32	0:00:25	0:00:46
	Carlingford _	Kent St	Beecroft Rd	0:03:49	0:03:24	0:01:27	0:01:36
3	Rd/Epping	Beecroft Rd	Blaxland Rd	0:01:10	0:01:30	0:00:45	0:01:07
	EB	Blaxland Rd	Essex St	0:00:43	0:00:40	0:00:25	0:00:35
		Essex St	Blaxland Rd	0:01:33	0:00:56	0:02:28	0:01:29
4	Carlingford Rd/Epping WB	Blaxland Rd	Carlingford Rd	0:00:47	0:01:05	0:01:19	0:01:29
5		Carlingford Rd	Kent St	0:00:47	0:00:42	0:00:50	0:00:48

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2.1.5 **Public Transport**

Table 2.2: Bus Routes

Route Number From/To							
Macquarie Park to Parramatta via Epping; Parramatta to Macquarie Park via Epping.							
Epping to Blacktown; Blacktown to Epping	DC.						
Epping to Eastwood; Eastwood to Epping.							
718w Epping Station, Beecroft Rd to James Ruse High School.	53						
Rouse Hill Station to Epping via Castle Hill; Epping to Rouse Hill Station via Castle Hill.							
Parramatta to Epping via Oatlands & North Rocks; Epping to Parramatta via Oatlands & North Rocks.	AC.						
Parramatta to Epping via North Rocks; Epping to Parramatta via North Rocks.	;;C \						
726w Epping Station, Beecroft Rd to Mount Saint Benedict High.							
732w Epping Station, Beecroft Rd to Baulkham Hills High.							
3013 Epping Station to Baulkham Hills HS.							
3014 Epping Station to Baulkham Hills HS.							
2.2 Site Visit							
Site visits were undertaken on the following occasions:							
25 October 2023, AM Peak 14 November 2023, PM Peak							
Photos from the site visits are presented in Appendix D . The main							

2.2 **Site Visit**

- 25 October 2023, AM Peak
- 14 November 2023, PM Peak

Photos from the site visits are presented in Appendix D. The main observations from the site visits are listed below.

2.2.1 **Congestion Locations**

The main areas of congestion in the AM Peak include:

- Carlingford Road eastbound approaching Ray Street and Beecroft Road
- Ray Street and Rawson Street approaches to Carlingford Road
- Beecroft Road southbound approaching Carlingford Road due to large weave movement across to the right lane, to turn into Blaxland Road.
- Essex Street approaching Epping Road.
- The westbound left turn lane in Epping Road, approaching Blaxland Road, is underutilised, whilst the adjacent through lanes experienced extensive queuing. Buses making a passenger stop on this approach, found it very difficult to merge back into the through lanes before Blaxland Road.
- 30min parking is allowed on Beecroft Rd NB.

The main areas of congestion in the PM peak include:

- Ray Street and Rawson Street approaches to Carlingford Road
- Essex Street approaching Epping Road.
- Blaxland Road northbound approaching Epping Road
- Bridge Street westbound approaching the Rawson Street roundabout.
- Parking is restricted on Beecroft Rd NB.

No school zones within the study area

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3 **Model Development**

3.1 **Modelling Platform**

Public Access Act 2009 The modelling analysis for this project is undertaken using VISSIM (Version 2023 SP02) and SIDRA Intersection analysis (Version 9.1).

3.2 **Model Development**

3.2.1 **Road Types**

The road network capacity and speeds are coded based on:

- Nearmap, spatially accurate aerial photography
- Roadway surveys
- Existing traffic signal layouts
- Existing speed limit zones including school zones, if any
- Site observations

3.2.2 **Traffic Demands**

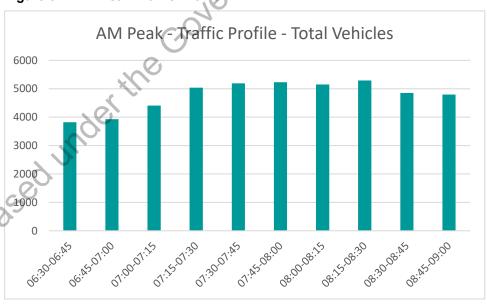
Existing traffic demands and patterns for the network were derived from turning volumes. The traffic profile for the AM and PM peak periods are illustrated in Figure 3.1 and Figure 3.2 respectively. Based on these following peak hours for AM and PM peak hours were determined and used for VISSIM and SIDRA modelling.

- 7:30am to 8:30am
- 5:00pm to 6:00pm

The VISSIM model includes a warm-up and cool-down period along with the peak hours:

- a 60-minute warm-up period
- a 30-minute cool-down period.

Figure 3.1: AM Peak Traffic Profile



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PM Peak - Traffic Profile - Total Vehicles Des: 6000 5000 4000 3000 2000 1000 Ω

Figure 3.2: PM Peak Traffic Profile

3.2.3 **Traffic Composition**

The Base Case matrices were developed for the following vehicle types

- Cars
- Trucks
- **Buses**

Additionally, pedestrian and cycle movements at crossing points are included in the VISSIM and SIDRA models.

The buses were modelled as a fixed route, as discussed in Section 0. The composition of vehicles in each peak hour is summarised in Table 3.1.

Table 3.1: Traffic Composition

Time Period	Cars	Trucks	Total
AM Peak		~,0	
07:30 -08:30	20,402	462	20,864
PM Peak	0		
17:00 -18:00	20,370	227	20,370

3.2.4 **Speed Profiles**

The models were developed using the VISSIM and SIDRA default values for driver behaviour and speed profiles for NSW, where available. Given that this is a low-speed environment, posted speed limits in the area are 60km/h or less, and the area is subject to congested traffic conditions, the respective default values in VISSIM and SIDRA are considered acceptable.

3.2.5 **Traffic Signals**

The VISSIM and SIDRA models were developed with fixed time signal operation. Signal timings, phases and offset data were derived from the SCATS data collected on the 21/9/2023. The SCATS timing data in Appendix H clearly shows that cycle and phase times remain very consistent during peak periods, so fixed time operation is justified.

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SCATS detector Counts, SCATS signal data, SCATS History and Controller information are presented in Appendices E, F, G and H. Existing traffic signal drawings are presented in Appendix I. Phase and cycle time of each signal are presented in Table 3.2.

Table 3.2: Average Signal Phase Time (s) - AM and PM Peaks

Intersection	Phase	AM Peak (07:30-08:30)	PM Peak (17:00-18:00)
	Α	86	84
Carlingford Rd & Ray Rd & Rawson St	В	44	46
	Total	129	130
	Α	47	57
Pagaroft Dd & Carlingford Dd	В	18	20
Beecroft Rd & Carlingford Rd	С	64	52
	Total	128	129
	Α	55	81
	В	23	23
Epping Rd & Blaxland Rd & Langston Pl	С	28	27
g	D	24	-
	Total	130	131
	Α	82	79
Enning Dd & Eggy St	В	27	27
Epping Rd & Essex St	С	21	23
	Total	130	128

3.2.6 **Public Transport**

Bus service arrivals and departures at Epping Station during the AM and PM peaks, to stands C, D, E, F and G were explicitly coded, based on service routes and timetables. Bus stands A and B, located in Cambridge Street on the east side of the station are excluded from the model.

The bus arrivals, dwell times and departure events are tabulated in Appendix J. The journey of each bus through the model was coded as a series of events (entries, bus stop dwells, layovers and/or exits).

Active Transport

Observed active transport crossing counts were included in all Base Case SIDRA and VISSIM models.

3.2.8 **Matrix Estimation**

Demand matrices for cars and trucks were generated at 15-minute intervals by applying the TFlowFuzzy methodology within the VISUM software. These matrices were derived from the observed turning traffic volumes (refer to Section 2.1.1), aiming to replicate the distribution of traffic demand and its changes throughout the modelling period. In this process, the 2021 STFM Base Case cordon matrices were utilized as the base travel pattern for the matrix estimation. The 2021 demand patterns are presented in Appendix

2023 Peak hour observed flows were coded directly into the Base case SIDRA models.

3.2.9 **Assignment Type**

Stochastic Route Choice was used to model the Epping Road in VISSIM. Given the lack of route choice available within the study area, the traffic assignment method effectively functions as 'all or nothing' assignment.

4 Model Calibration and Validation

4.1 Overview

The model calibration involves aligning observed traffic conditions with the modelled traffic conditions including vehicle behaviour observed on site. Turning counts and queue length, travel time have been selected as key performance indicators (KPI's) of model calibration for VISSIM Modelling and queue length for SIDRA modelling. Data collected from traffic surveys, as discussed in **Section 2.1** has been compared against model outputs. Furthermore, model stability has been tested against a set of criteria outlined below.

4.2 Model Calibration VISSIM Modelling

4.2.1 Model Calibration Criteria

This model is calibrated against the core area criteria:

- Core model tolerance criteria listed in Table 4-1.
- 100 per cent of observations to be within tolerance limits.
- R² value of observed versus modelled plots to be >0.95
- Average modelled cycle time for each 1-hour period to be within 10% of observed SCATS cycle time for the same period
- Total green-time over each 1-hour period to be within 10% of observed SCATS equivalent for each phase

Table 4-1 Core Area Tolerance Limits

Flow	Criteria
< 99	To be within 10 vehicles of observed
100-999	To be within 10% of observed
1000-1999	To be within 100 vehicles of observed
>2000	To be within 5% of observed

4.2.2 Turning Count Calibration Results

Table 4-1 describes the network wide criteria and measurement requirements set out by TfNSW in the Traffic Modelling Guidelines (2013) for turning and link counts.

The traffic flows for each modelled hour were calibrated against the observed turning counts. The calibration outcomes for the AM and PM peak periods are summarised in **Table 4.2** and **Table 4.3** respectively.

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Table 4.2: Base Model Turning Flows Calibration - AM Peak

AM Peak (07:30-08:30) - Calibration Results

Plot	Results				
Percentage of turn/link volumes	75/77 (97%)				
R-square value	1.00				

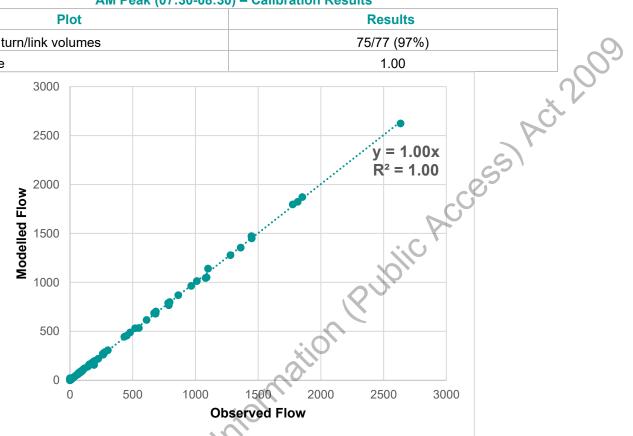


Table 4.3: Base Model Turning Flows Calibration - PM Peak

Plot		Results				
centage of turn/link volumes			75/77 (97%)			
quare value	70,			1.00		
2500 2000 MOI 1500 Pelle pow 1000 500	500	1000 Observed	1500 I Flow	y = 1.00x R ² = 1.00		

Based on the above results, the Base Case models of Epping Road bridge were well calibrated using the turning movements at each intersection and all cordon entry/exit points against the observed traffic turning counts. However, there are two turning fail to meet the criteria due to traffic imbalances created by Hunts La street.

4.2.3 Signal Time Calibration Results

A comparison of observed and modelled signal times at each intersection is presented Table 4-4.

During the AM and PM peaks, the modelled average cycle time and phase times shows a good match with the observed times (within 10%).

Table 4-4 Signal Time Comparison – AM and PM Peaks

lutava ati au	Dhasa	AM P	eak (07:30-08	3:30)	PM Peak (17:00-18:00)			
Intersection	Phase	Observed	Modelled	Diff %	Observed	Modelled	Diff %	
Carlingford Rd &	Α	86	83	3%	84	84	0%	
Ray Rd & Rawson St	В	44	47	-7%	46	46	0%	
	Total	129	130	-1%	130	130	0%	
	Α	47	47	0%	57	57	0%	
Beecroft Rd & Carlingford Rd	В	18	18	-3%	20	21	-8%	
	С	64	64	-1%	52	52	0%	
	Total	128	129	-1%	129	130	-1%	
Epping Rd & Blaxland Rd &	Α	55	55	1%	81	81	0%	
	В	23	25	-7%	23	24	-2%	
Blaxland Rd &	С	28	29	-2%	27	26	6%	
Langston PI	D	24	23	6%	-	-	-	
	Total	130	130	0%	131	130	1%	
	Α	82	81	2%	79	77	3%	
Epping Rd &	В	27	29	-7%	27	29	-9%	
Essex St	С	21	21	2%	23	25	-8%	
	Total	130	130	0%	128	130	-1%	

4.2.4 Model Stability

The model statistics for assessing calibration and validation were based on a median run selected from five model seed values (560, 28, 7771, 86524 and 2849). An analysis of VHT and VKT results for the five seed values was used to demonstrate model stability.

4.2.5 Demonstration of Model Stability

The ability of a model to produce consistent results for several seed values is referred to as model stability. The stability of the model is assessed by comparing the total vehicle hours travelled (VHT) network performance indicator between the five model seed runs undertaken for each period.

Based on this, and as indicated in the TfNSW Traffic Modelling Guidelines (2013), the representative seed run is selected (median VHT value). All results reported are based on this single representative seed run. As such, **Figure 4.1** and **Figure 4.2** illustrate the VHT during the AM and PM peak periods respectively.

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Figure 4.1: Total Vehicle Hours Travelled (VHT) - AM Peak

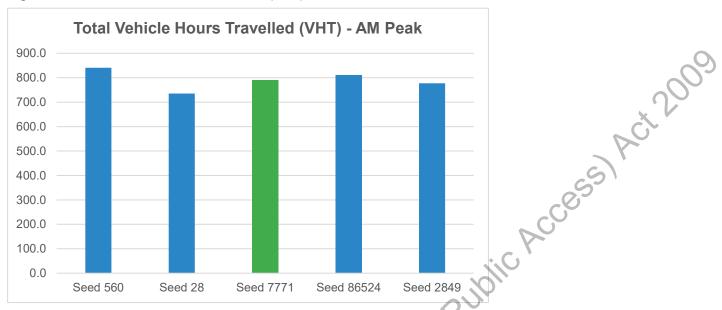
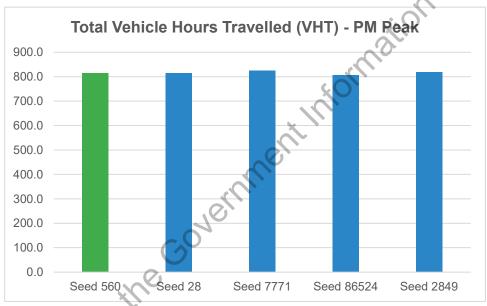


Figure 4.2: Total Vehicle Hours Travelled (VHT) - PM Peak



The difference between the results produced by each seed was within 7% and 1% in the AM and PM peaks compared to the respective median values. Hence, the AM and PM peak modelling show that both models are stable. The median seed value (in green) for AM Peak is 7771 and for PM Peak the median seed value is 560.

4.2.6 Model Validation

VISSIM Model validation has been undertaken using observed travel time data. The objective is to achieve modelled travel times within 15% of observed travels times, or within 1-minute, whichever is the highest.

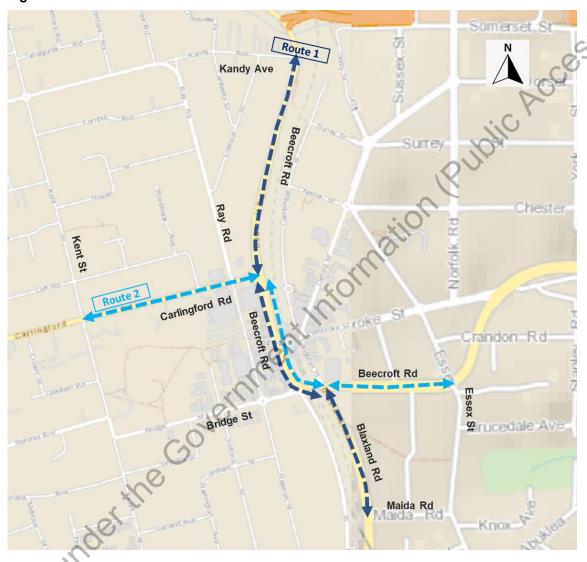
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4.2.7 Travel Time Validation

The model validation has been undertaken based on the observed travel time (Refer to **Section 2.1.4** for details). Travel time data has been collected during AM and PM peak periods for two routes within the model area which are shown in **Figure 4-3** and listed below:

- Route 1: Beecroft Road, Blaxland Road (Northbound and Southbound)
- Route 2: Carlingford Road, Beecroft Road and Epping Road (Eastbound and Westbound)

Figure 4-3 Travel Time Routes



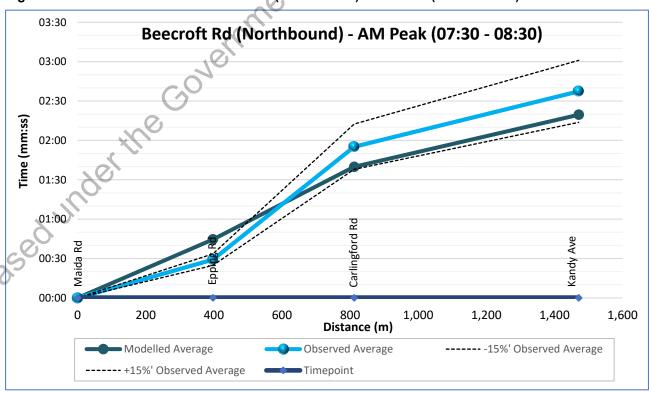
The travel time comparison for the above-mentioned routes are provided in **Table 4-5**. Also, the cumulative travel time graphs for each route are provided in **Figure 4-4** to **Figure 4-11**.

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Table 4-5 Travel Time Validation Results - AM and PM Peaks

Devite	-	.	AM Pe	AM Peak (07:30-08:30) PM Peak (17:00-18				:00)	
Route	From	То	Observed	Modelled	Diff %	Observed	Modelled	Diff %	
Beecroft Rd	Maida Rd	Epping Rd	00:29	00:45	-53%	03:06	01:11	62%	
	Epping Rd	Carlingford Rd	01:26	00:55	36%	01:23	00:57	31%	
(Northbound)	Carlingford Rd	Kandy Ave	00:42	00:40	6%	00:40	00:40	2%	
	Overall		02:37	02:20	11%	05:10	02:48	46%	
	Kandy Ave	Carlingford Rd	01:04	01:04	1%	01:18	00:54	31%	
Beecroft Rd	Carlingford Rd	Blaxland Rd	02:54	02:34	11%	06:09	07:00	-14%	
(Southbound)	Blaxland Rd	Maida Rd	00:25	00:25	-1%	00:25	00:25	1%	
	Overall		04:23	04:03	8%	07:52	08:19	-6%	
	Kent St	Beecroft Rd	03:49	03:34	6%	01:27	01:30	-4%	
Carlingford Rd/Epping	Beecroft Rd	Blaxland Rd	01:10	00:54	23%	00:45	00:40	11%	
Rd (Eastbound)	Blaxland Rd	Essex St	00:43	00:48	-11%	00:25	00:42	-65%	
	Overall		05:42	05:16	8%	02:37	02:52	-10%	
	Essex St	Blaxland Rd	01:33	01:32	1%	02:28	01:20	46%	
Carlingford Rd/Epping	Blaxland Rd	Carlingford Rd	00:47	00:49	-3%	01:19	01:07	15%	
Rd (Westbound)	Carlingford Rd	Kent St	00:47	00:46	2%	00:50	00:46	8%	
	Overall		03:07	03:07	0%	04:36	03:13	30%	

Figure 4-4 Travel Time Plot - Beecroft Rd (Northbound) - AM Peak (07:30 - 08:30)



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Figure 4-5 Travel Time Plot - Beecroft Rd (Southbound) - AM Peak (07:30 - 08:30)

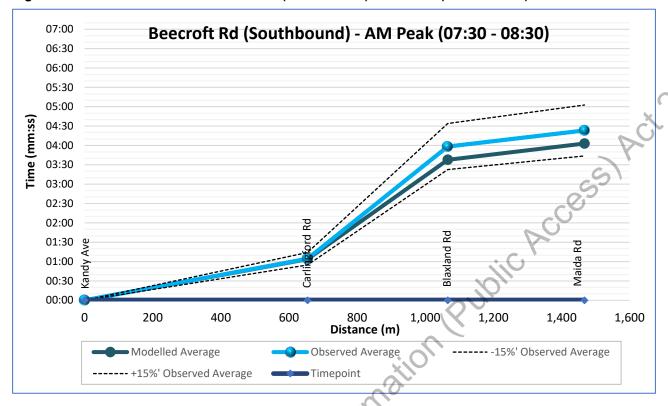
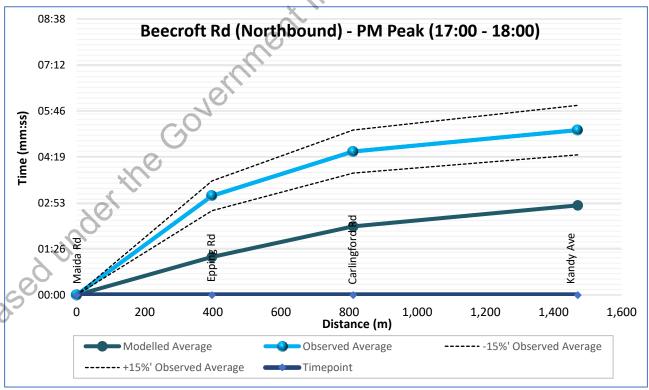


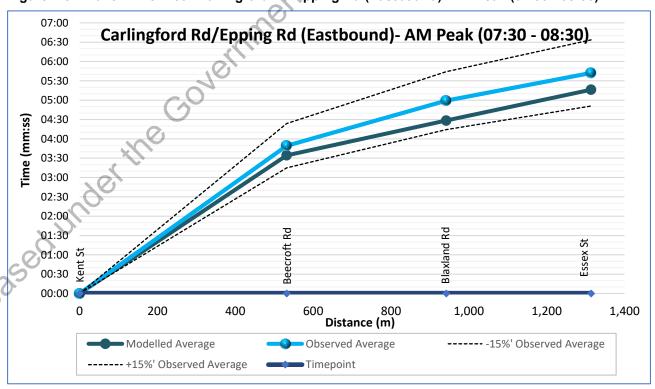
Figure 4-6 Travel Time Plot - Beecroft Rd (Northbound) - PM Peak (17:00 - 18:00)



10:05 Beecroft Rd (Southbound) - PM Peak (17:00 - 18:00) 08:38 07:12 Time (mm:ss) 05:46 04:19 02:53 01:26 00:00 1,000 200 600 800 1,400 400 1,200 1,600 Distance (m) Modelled Average Observed Average --- -15%' Observed Average - +15%' Observed Average

Figure 4-7 Travel Time Plot - Beecroft Rd (Southbound) - PM Peak (17:00 - 18:00)





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Figure 4-9 Travel Time Plot - Carlingford Rd/Epping Rd (Westbound)- AM Peak (07:30 - 08:30)

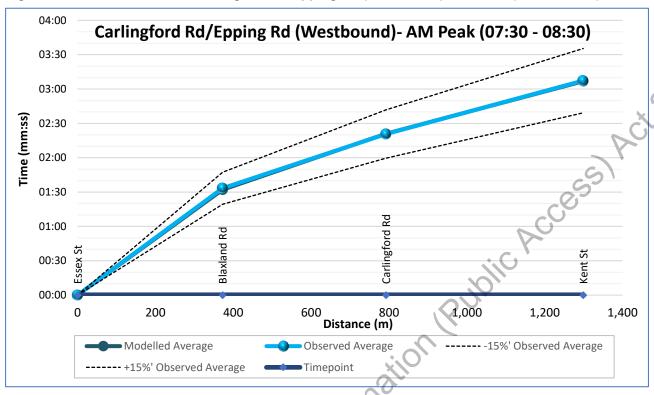
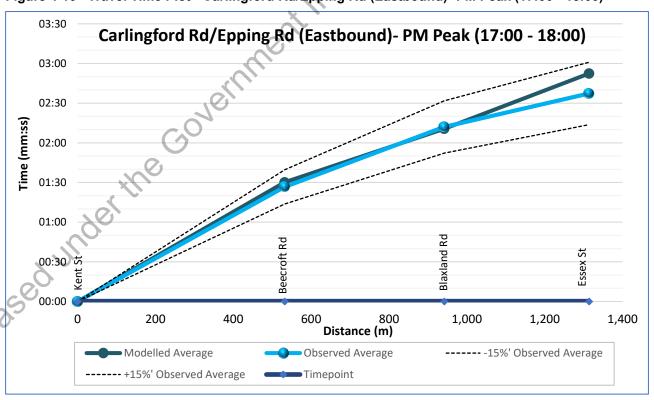


Figure 4-10 Travel Time Plot - Carlingford Rd/Epping Rd (Eastbound) - PM Peak (17:00 - 18:00)



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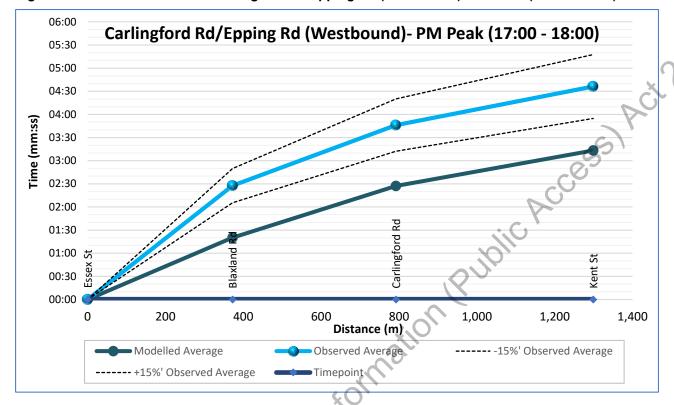


Figure 4-11 Travel Time Plot - Carlingford Rd/Epping Rd (Westbound) - PM Peak (17:00 - 18:00)

4.2.7.1 Key Finding

- The analysis reveals that most travel times align well with the observed data, with most sections differing by less than 15%. Instances where the threshold is exceeded typically involve differences of less than 25 seconds, mainly in shorter or relatively low travel time sections.
- During the PM peak, Beecroft Rd (Northbound) and Carlingford Rd/Epping Rd (Westbound) are noted as
 not meeting the criteria due to long observed travel times on individual sections within the overall survey
 route. The uncharacteristically long travel times in these segments seemed inconsistent with travel times
 in adjacent time periods, the observed traffic queueing and the short travel distances involved. Such
 inconsistencies can occur when the number of data samples is low. The data may have been influenced
 by a specific traffic event, data collection errors, or the behaviour of the surveyor (driver).
- To check the perceived inconsistencies in the floating car surveys, a secondary data source was used to check the validation. HERE data was sourced, for the same period, to check the segments in question. The HERE data provides a larger sample of travel times in those segments and confirmed that the floating car data was uncharacteristically high. The modelled flows were consistent with the HERE travel time data.
- Based on the review against multiple travel time validation data sources, the analysis confirmed that the modelled travel times are consistent with observed travel times and that the model is well calibrated and validated.

4.2.8 Queue Length Validation

A summary of modelled and observed average queue lengths (vehicles) for each approach are provided in **Table** 4-6.

Table 4-6 Average Queue Length (meters) Validation Summary - AM and PM Peaks

	•	• •	•		•				
Intersection	Dhaca	AM P	eak (07:30-08	3:30)	PM P	PM Peak (17:00-18:00)			
	Phase	Observed	Modelled	Diff	Observed	Modelled	Diff		
	South	27	21	-6	77	17	-60		
Carlingford Rd &	East	40	11	-29	46	16	-30		
Ray Rd & Rawson St	North	63	37	-26	67	34	-33		
	West	292	234	-58	106	40	-66		
	South	97	25	-72	129	59	-70		
Beecroft Rd &	East	-	-	-	-	. ċ. Y	-		
Carlingford Rd	North	62	24	Billed Diff Observed Modelled Diff 1 -6 77 17 -60 1 -29 46 16 -30 7 -26 67 34 -33 34 -58 106 40 -66 5 -72 129 59 -70 - - - - - 4 -38 40 14 -26 0 -34 60 19 -41 0 -5 4 0 -4 - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -	-26				
	West	54	20	-34		19	-41		
	South	5	0	-5	4	0	-4		
Beecroft Road / High	East	-	-	-	7	-	-		
Street / Bridge Street	Beecroft Road / High Street / Bridge Street East -<	-	:(0)-	-	-				
ou oot	West	23	5	-18	26	26 7 -	-19		
	South	18	2	-16	18	4	-14		
Bridge Street / Rawson	East	23	1	-22	33	2	-31		
Street	North	26	12	-14	21	4	-17		
	West	23	8	-15	18	2	-16		
	South	38	20	-18	244	42	-202		
Epping Rd &	East	orth 63 37 -26 67 34 /est 292 234 -58 106 40 outh 97 25 -72 129 59 asst - - - - orth 62 24 -38 40 14 /est 54 20 -34 60 19 outh 5 0 -5 4 0 asst - - - - - orth - - - - - - /est 23 5 -18 26 7 - outh 18 2 -16 18 4 asst 23 1 -22 33 2 orth 26 12 -14 21 4 /est 23 8 -15 18 2 outh 38 20	-66						
Langston Pl	North	31	12	-19	30	13	-60 -30 -33 -66 -7026 -41 -419 -14 -31 -17 -16 -202 -66 -17 -39		
	West	145	145 157 12 204 165	165	-39				
Street North 26 12 West 23 8 South 38 20 Epping Rd & Blaxland Rd & Langston Pl East 113 75 North 31 12 West 145 157 South 13 12 Epping Rd & East 10 7	12	-1	22	15	-7				
	East	10	7	-3	131	19	-112		
	North	39	47	8	50	101	51		
	West	88	74	-14	76	19 0 - - 7 4 2 4 2 42 98 13 165 15	-51		

4.2.8.1 Key Findings

- In the AM peak, the modelled queue lengths are generally a good match to the observed queue lengths. The largest AM Peak average queue length inconsistencies occurred on the southern approach to the Beecroft Rd / Carlingford Rd intersection. The modelled queues were slightly lower than modelled.
- In the PM peak, the modelled queue lengths are generally a good match to the observed in most of the approaches. The largest PM Peak average queue length inconsistencies occurred on the southern approach to Epping Road / Blaxland Road / Langston Place intersection. The modelled queues were lower than the observed.

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4.2.9 **Network Performance**

The network performance statistics indicate the efficiency of road network. For instance, models showing a comparatively higher level of vehicle stops typically represent high levels of congestion, which generally cess Act 200° reduce throughput and increase greenhouse emissions (due to the increased stop/start nature of the driving). Conversely, a comparatively low VHT typically represents a network with a relatively low level of congestion and is generally be correlated with a low level of vehicle stops.

Table 4-7 presents the key network performance parameters for the following:

- Total distance travelled by all vehicles within the modelled network (VKT);
- Total travel time by all vehicles within the modelled network (VHT); and
- The average speed of vehicle trips on all roads within the microsimulation model;
- The average delay of vehicle trips on all roads within the microsimulation model;
- The total completed vehicle trips on all roads within the microsimulation model;
- Unreleased trips (Latent Demand) demand that could not be released in the peak hour;
- The average number of vehicle stops on all roads within the microsimulation models.

Table 4-7 Network Performance Summary - AM and PM Peaks

Model Performance Parameters	AM Peak (07:30-08:30)	PM Peak (17:00-18:00)
Vehicle kilometres travelled (VKT) km	9651	9563
Vehicle hours travelled (VHT) Hours	369	345
Average network speed (km/h)	26	28
Average network delay (s)	111.9	97.5
Completed trips	6119	6252
Unreleased trips	5	0
Number of vehicle stops Average	3.12	2.29

The average network speeds are comparatively lower in both AM and PM peaks. This indicates the increased congestion levels with the future demand growth.

4.2.10 Intersection Performance

Table 4.8 summarises approach and overall intersection performance in terms of Average queue, delay and LOS based on the TfNSW method, indicated below.

LOS	Delay	(sec)
	Low	High
Α	0	14
B	15	28
S C	29	42
D	43	56
Е	57	70
F	>	71

Table 4.8: Intersection Performance – AM and PM Peak

AM Peak (07:30-08:30)

PM Peak (17:00-18:00)

Intersection	Approach	Traffic Volume	Delay (s)	LOS	Average Queue (m)	Traffic Volume	Delay (s)	LOS	Average Queue (m)
	South	269	73	F	21	356	50	D	17
Carlingford Rd	East	876	9	Α	11	1,184	8	Α	16
& Ray Rd &	North	483	62	Е	37	234	40	C	10
Rawson St	West	1098	146	F	234	952	50	D	40
	Overall	2,726	80	F	107	2,726	31	(6)	24
	South	1,668	31	С	25	2,168	34	C)C	59
Beecroft Rd &	North	1,452	31	С	24	1,194	32	C	14
Carlingford Rd	West	1,410	10	Α	20	1,061	14	Α	19
	Overall	4,530	25	В	23	4,423	28	В	37
:	South	191	13	Α	2	196	16	В	4
Bridge Street	East	198	9	Α	1	369	8	Α	2
/ Rawson Street	North	293	26	В	12	230	13	Α	4
Street	West	305	15	В	8	174	9	Α	2
	Overall	987	41	С	12	969	45	D	4
Beecroft	South	24	9	Α	0	17	11	Α	0
Road / High	East	1716	1	Α	0	2,500	1	Α	0
Street / Bridge Street	West	207	12	А	5	128	26	В	7
	Overall	1,947	12	Α	5	2,645	26	В	7
	South	794	27	В	20	977	47	D	42
Epping Rd &	East	1,118	54	D	75	1696	48	D	98
Blaxland Rd &	North	265	49	D	12	242	54	D	13
Langston PI	West	2,640	42	С	157	1,867	57	D	165
	Overall	4,817	43	С	107	4,782	52	D	108
	South	185	57	E	12	293	53	D	15
	East	690	14	Α	7	1,202	21	В	19
Epping Rd & Essex St	North	519	74	F	47	562	120	F	101
	West	1843	25	В	74	1,121	20	В	25
2	Overall	3,237	32	С	52	3,178	41	С	35

- Road / Ray Road / Rawson Street intersection, in the AM peak.

 The north approach at the Epping Road / Essex Street and PM peaks, South approach All the intersections perform satisfactorily at LOS D or better in both AM and PM peaks except Carlingford
 - The north approach at the Epping Road / Essex Street intersection is performing at LOS F in both AM and PM peaks, South approach in the AM peak operates with LOS E.

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4.3 Model Calibration SIDRA Modelling

4.3.1 Calibration Process

The following parameters have been changed to calibrate the surveyed queue lengths recorded on-site.

Carlingford Road / Ray Road / Rawson Street

Blocking Calibration

- This includes filtering effects in the B phase where vehicles clear at the end of the phase.
- Blocking calibration to reduce queueing from Ray Road (0.2 and 1.8 for AM and PM respectively)
- Blocking calibration to reduce queueing from Rawson Street (0).
- Blocking calibration to increase queueing from the West (1.04).

Lane Utilisation

- 90% Lane Utilisation on the kerbside lane from the south approach.
- West appr adjusted coordination arrival type to 5 to reduce the blockage right turn from Beecroft Road.

Phase Timing

- SCATS History shows a fixed split plan configured during peaks.
- It is assumed a priority green split was locked given to Carlingford Road over the side streets.

Beecroft Road and Carlingford Road

Signal coordination calibration

- Increased the rating to 6 and 85% arrived during Green on the Southern approach (AM).
- Increased the rating to 6 and 86% arrived during Green on the Southern approach (PM).

Epping Road / Blaxland Road / Langston Place

Signal coordination calibration

- AM peak increased the Arrival during Green to 70% on the east approach.
- PM peak increase the Arrival during Green to 80% on the east approach.

Lane Capacity Adjustment

 Increased the south approach queue via capacity reduction, -58%. It is assumed that the queue did not dissipate before the PM peak due to the split plan locked from 3:15 pm

Phase Timing

- SCATS History shows a fixed split plan configured during peaks.
- It is assumed a priority green split was locked given to Epping Road over the side streets.

Epping Road / Essex Street

Signal coordination calibration

AM peak - increased the rating to 6 with coordination on the west approach.

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PM Peak - increase the % arrival during green for westbound to 80% and reduced the arrival rating to 2
on the east approach.

Lane Utilisation

- Lane 1 utilisation east approach is adjusted with lane-by-lane queue length. 5%
- Lane 1 utilisation south approach is adjusted with lane-by-lane queue length. 95%

Beecroft Road / High Street / Bridge Street

Lane Capacity - The capacity on the High Street (left into Beecroft Road) was reduced to 70%. It is due to limited sight visibility and high speed on the main road.

4.3.2 Signal Time Calibration Results

Like VISSIM modelling, a comparison of observed and Sidra modelled signal times at each intersection is presented **Table 4-9**.

Table 4-9 Sidra Signal Time Comparison - AM and PM Peaks

AM Peak (07:30-08:30) PM Peak (17:00-18:00) Intersection **Phase** Observed Modelled Diff % Observed Modelled Diff % Α 86 85 1% 84 85 -1% Carlingford Rd В 45 -2% 2% & Ray Rd & 44 46 45 Rawson St Total 129 130 -1% 130 130 0% Α 47 9% 57 58 -2% 43 В 18 16 13% 20 20 0% Beecroft Rd & Carlingford Rd 71 С 64 -10% 52 52 0% -1% Total 128 130 -2% 130 129 Α 55 56 2% 81 81 0% В 23 0% 0% 23 23 23 Epping Rd & Blaxland Rd & С 28 28 0% 27 26 4% Langston PI D 24 24 0% Total 0% 130 1% 129 130 131 A 82 -1% 80 2% 79 80 В 27 27 0% 27 27 0% Epping Rd & Fssex St -9% 0% С 21 23 23 23

4.3.2.1 Key Findings

Total

 During the AM and PM peaks, the modelled average cycle time and phase times shows a good match with the observed times (within 10%).

0%

130

- The only exception is phase B during the AM Peak at Beecroft Road and Carlingford Road, where the absolute difference is only 2 seconds.
- The modelled signal timings are considered well calibrated with observed.

130

-2%

130

128

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4.3.3 Queue Length Calibration Results

A summary of modelled and observed average queue lengths (in metres) for each approach is provided in **Table 4-10.** The modelled queue lengths for both peaks show a good match with the observed queues.

It is noted that the observed queue length comparison for VISSIM models are different because the approach queue length is an average across all the lanes. In contrast the SIDRA's approach queue length is taken from the maximum queue length of all the lanes. The observed queue length calculation is consistent with SIDRA outputs.

Table 4-10 Sidra Output - Averaged Queue Length Comparison (metres)

		AM Peak (0	7:30-08:30)	PM Peak (17:00-18:00)		
Intersection	Approach	Observed	Modelled	Observed	Modelled	
	South	35	49	91	75	
Carlingford Rd & Ray	East	42	49	49	40	
Rd & Rawson St	North	84	92	98	91	
	West	294	257	119	137	
	South	98	87	147	125	
Beecroft Rd &	East	-	- 0	-	-	
Carlingford Rd	North	63	105	49	74	
	West			50		
	South	7	0	7	0	
Beecroft Road / High	East	-	· O() -	-	-	
Street / Bridge Street	North	-	-	-	-	
	West	21	9	28	20	
Bridge Street	South	21	11	28	12	
/ Rawson	East	28	11	49	24	
Street	North	21	19	28	12	
	West	21	18	28	6	
	South	49	52	252	185	
Epping Rd & Blaxland	East	112	127	182	137	
Rd & Langston PI	North	35	45	- 28 2 28 1 49 2 28 1 28 1 28 1 28 1 38 6 252 18 182 13 35 4 203 13	43	
< X	West	126	183	203	139	
76,	South	35	22	35	47	
Enning Dd & Essay Ct	East	21	35	210	136	
Epping Rd & Essex St	North	49	82	63	82	
CO.	West	84	69	77	51	

4.3.4 Intersection Performance

The Sidra outputs of the base model, AM and PM peaks, are shown in **Table 4.11**.

Table 4.11: Intersection Performance – AM and PM Peak

			AM Peak (07:30-08:30)				PM Peak (17:00-18:0	00)
Intersection	Approach	Traffic Volume	Delay (s)	LOS	Avg Queue (m)	Traffic Volume	Delay (s)	LOS	Avg Queue (m)
_	South	267	53	D	58	350	170	F	53
Carlingford Rd	East	898	6	Α	38	1200	3	Α	40
& Ray Rd & Rawson St	North	494	83	F	52	229	372	F	37
Rawson St	West	1064	107	F	213	933	36	С	137
	Overall	2723	64	E	213	2711	67	E	137
	South	1695	9	Α	120	2189	10	A	137
Beecroft Rd &	North	1467	41	С	43	1199	46	D	74
Carlingford Rd	West	1413	48	D	50	1070	44	D	50
	Overall	4575	31	С	120	4458	28	В	137
_	South	196	18	В	m	191	20	В	12
Dridge Street	East	222	18	В	15	377	120	F	24
Bridge Street / Rawson Street	North	344	20	В	14	288	6	Α	13
Sireei	West	326	10	Α	12	190	5	А	6
•	Overall	1088	18	В	14	1047	49	D	24
	South	26	5	Α	0	18	6	Α	0
Beecroft	East	1701	4	NA	0	2530	4	NA	0
Road / High Street / Bridge	North	2582	6	NA	109	1875	6	NA	76
Street	West	156	19	В	8	146	64	E	20
•	Overall ¹	4468	19	В	8	4570	64	E	20
	South	803	31	С	49	976	106	F	38
	East	1165	20	В	123	1731	10	А	137
Epping Rd & Blaxland Rd &	North	264	59	Е	42	262	58	E	43
Langston PI	West	2650	48	D	172	1881	43	С	139
	Overall	4881	36	С	123	4850	42	С	185
71.	South	183	74	F	26	280	103	F	15
-9	East	712	15	В	30	1203	34	С	136
Epping Rd & Essex St	North	536	93	F	72	569	91	F	82
	West	1874	5	Α	70	1098	8	Α	51
•	Overall	3307	25	В	72	3149	42	С	136

¹ Level of Service (LOS) of worst movement

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4.3.4.1 **Key Findings**

- All the intersections perform satisfactorily at LOS D or better in both peaks except for the intersection of
- a software will be used as software will be use

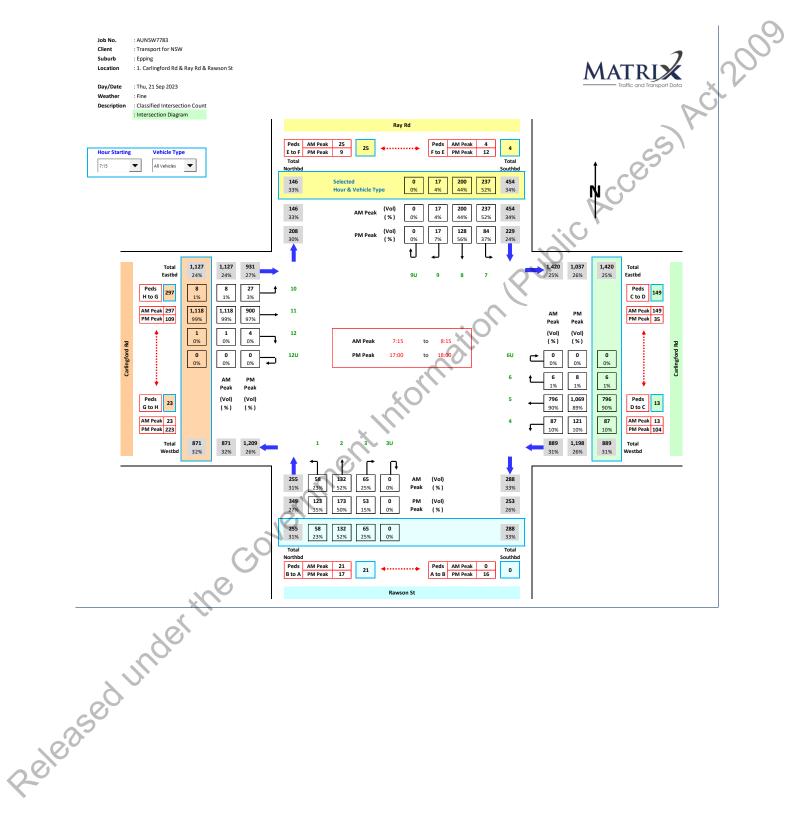
5 Summary and Conclusion

The purpose of this modelling was to develop a calibrated and validated 2023 base model for VISSIM and SIDRA models for the Epping Bridge project. The base model was developed to represent the typical 2023 weekday AM and PM peak periods.

The AM and PM peak models are suitably calibrated for the 2023 weekday AM and weekday PM peak periods. Comparing the turning movement counts in the VISSIM model to the observed counts, it was concluded that the models conform with the calibration criteria set out in the TfNSW Traffic Modelling Guidelines (2013) for microsimulation models.

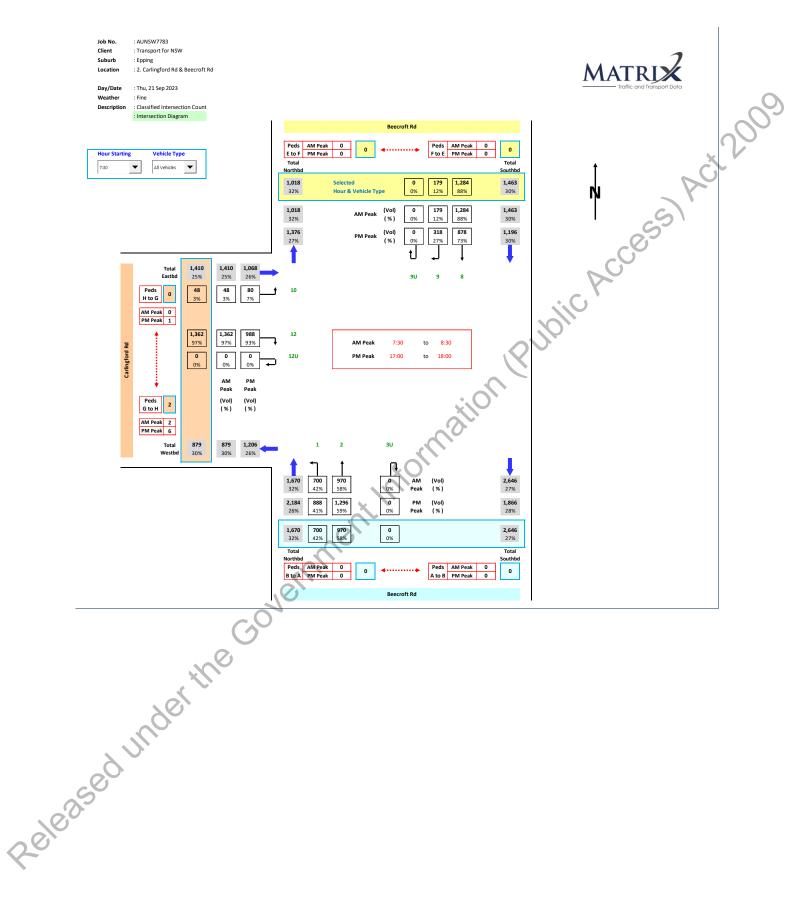
Aleased under the Government into mation Ruder the Government into mation Ruder the Robert into the state of the content of th The queue lengths and travel times are satisfactorily validated against the observed queues recorded on site for VISSIM modelling. The SIDRA Model was validated against observed queue lengths, and it found to be satisfactory. The 2023 AM and PM Peak base models were therefore considered fit-for-purpose for use in

A. Classified Intersection Traffic Counts (21/9/2023)

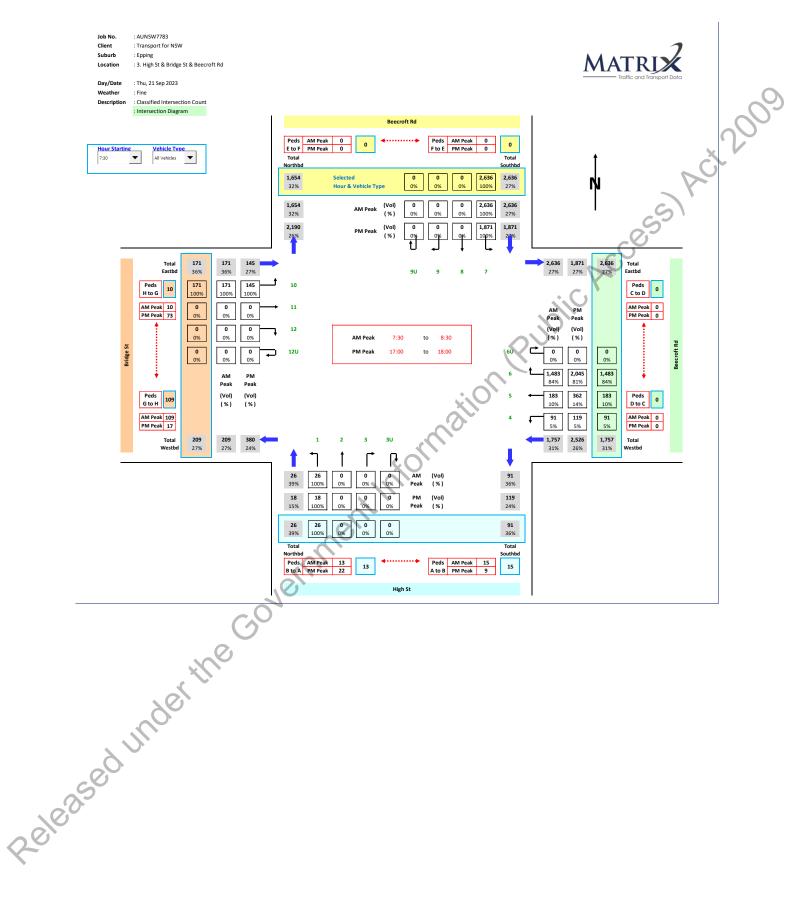


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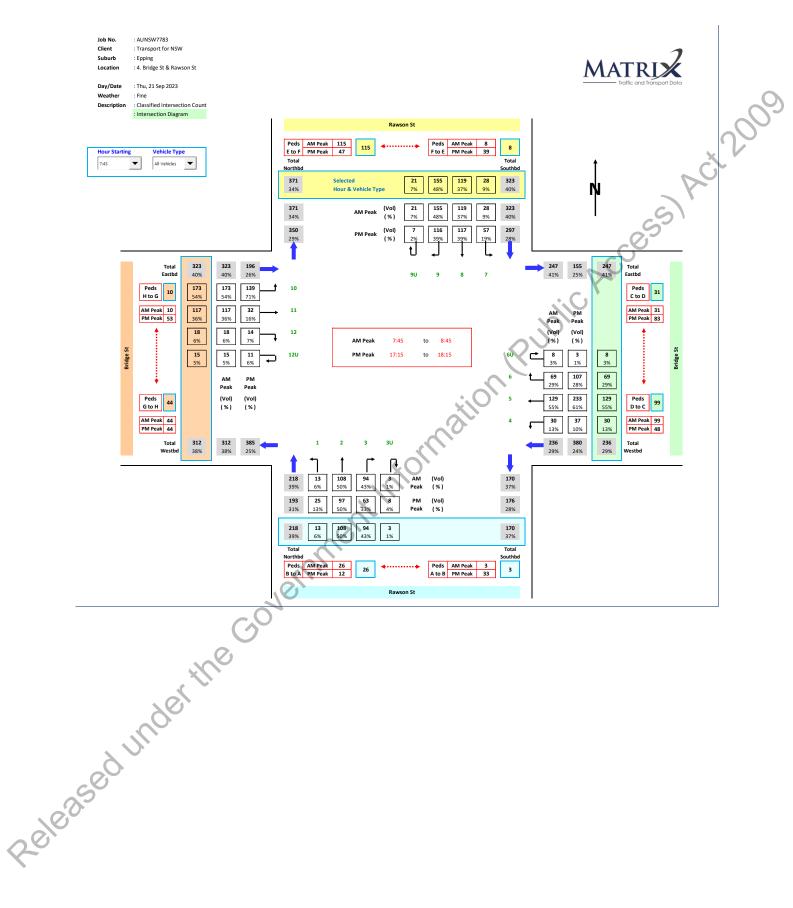
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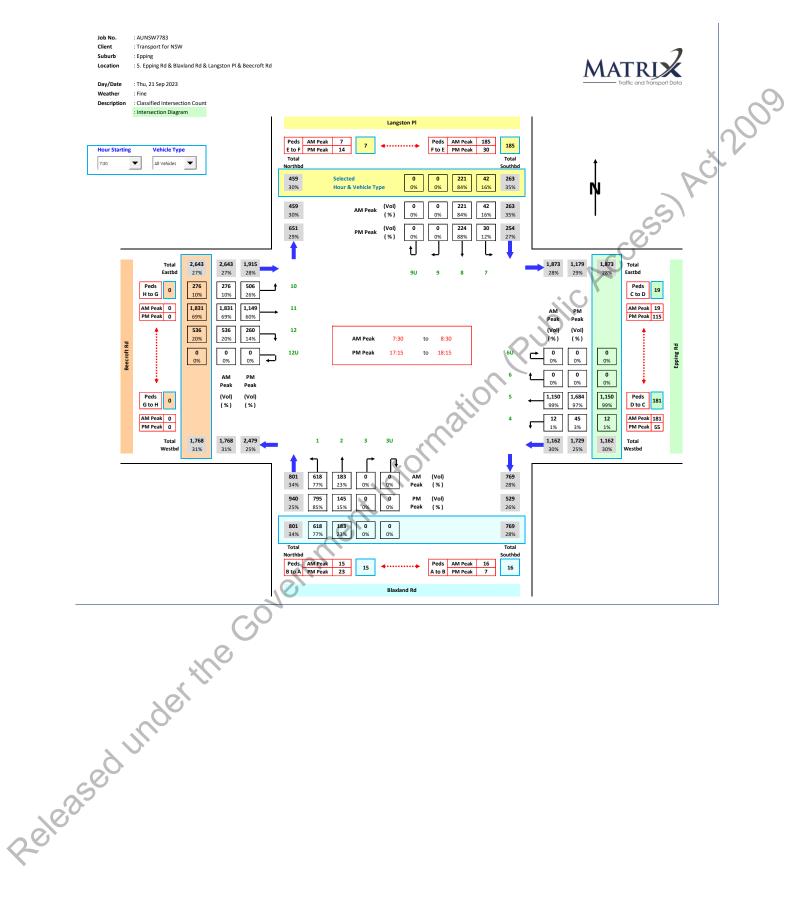


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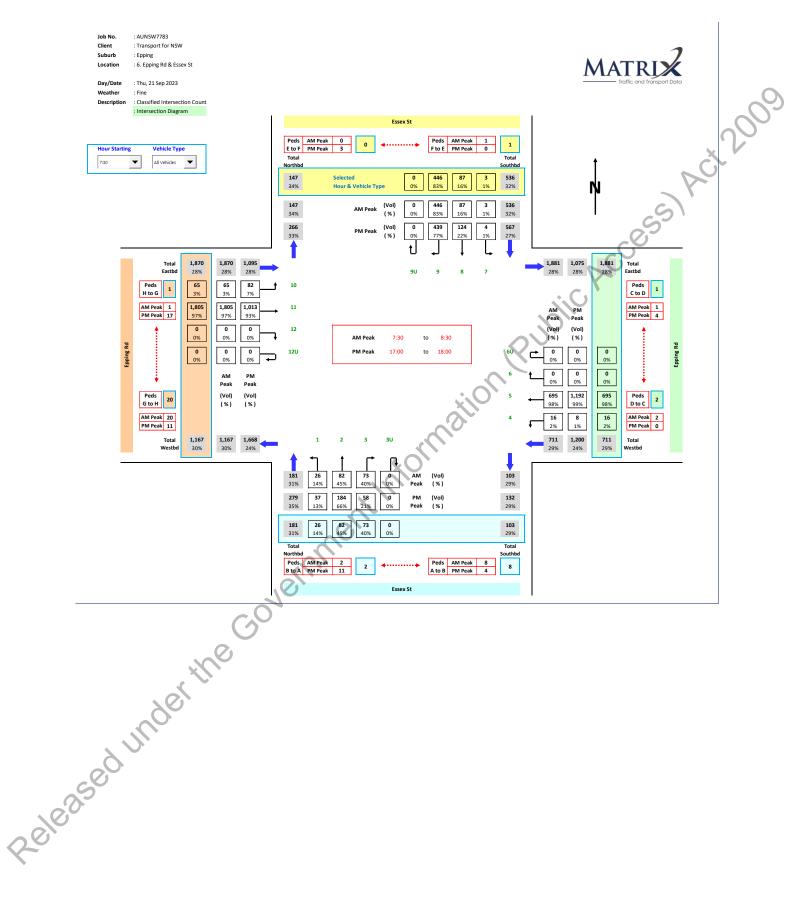


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B. Average Queue Length Data (21/9/2023)

Table B1: Carlingford Road, Ray Road and Rawson Street, Average Queue Length (Vehicles)

Table B1: Carlingford Road, Ray Road and Rawson Street, Average Queue Length (Vehicles) 15-min Period Starting	6:30 6:45 7:00 7:15 7:30 7:45 8:00 8:15 8:30 8:45 16:00 16:15	1 2 3 4 4 6 6 5 6	2 3 3 4 3 3 4	2 2 2 5 7	1 2 2	3 4	2 2	10	Lane 2	
8:30	6:45 7:00 7:15 7:30 7:45 8:00 8:15 8:30 8:45 16:00 16:15	2 3 4 4 6 6 5 6	3 3 4 3 3 4	2 2 5 7	2 2	4	2		10	
8:30	7:00 7:15 7:30 7:45 8:00 8:15 8:30 8:45 16:00 16:15	3 4 4 6 6 5 6	3 4 3 3 4	2 5 7	2	-		10		Č
8:30	7:15 7:30 7:45 8:00 8:15 8:30 8:45 16:00 16:15	4 4 6 6 5 6	4 3 3 4	5 7		4	Τ ,	12	10	D'
8:30	7:30 7:45 8:00 8:15 8:30 8:45 16:00 16:15	4 6 6 5 6	3 3 4	7	4		3	10	10	
8:30	7:45 8:00 8:15 8:30 8:45 16:00 16:15	6 6 5 6	3 4			6	5	18	19	55,
8:30	8:00 8:15 8:30 8:45 16:00 16:15	6 5 6	4		7	9	7	40	38	
8:30	8:15 8:30 8:45 16:00 16:15	5 6		6	6	11	7	40	42	70
8:30	8:30 8:45 16:00 16:15	6	E	6	6	11	7	47	49	. ~
8:30	8:45 16:00 16:15		5	7	5	17	6	40	39)
16:00 10 14 6 7 9 4 7 6 16:15 8 14 6 6 9 3 11 8 16:30 7 14 7 7 6 4 9 6 16:45 8 14 6 6 5 2 10 9 17:00 11 14 7 7 15 4 23 20 17:15 7 14 6 6 20 6 17 12 17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	16:00 16:15	_	10	8	6	21	6	33	30	
16:15 8 14 6 6 9 3 11 8 16:30 7 14 7 7 6 4 9 6 16:45 8 14 6 6 5 2 10 9 17:00 11 14 7 7 15 4 23 20 17:15 7 14 6 6 20 6 17 12 17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 For each of the property Data, 21/9/2023, 6am-10am and 3pm-7pm	16:15	5	5	7	6	19	7	17	16	
16:30 7 14 7 7 6 4 9 6 16:45 8 14 6 6 5 2 10 9 17:00 11 14 7 7 15 4 23 20 17:15 7 14 6 6 20 6 17 12 17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm		10	14	6	7	9	4	7	6	
16:45 8 14 6 6 5 2 10 9 17:00 11 14 7 7 15 4 23 20 17:15 7 14 6 6 20 6 17 12 17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	16:30	8	14	6	6	9	3		8	
17:00 11 14 7 7 15 4 23 20 17:15 7 14 6 6 20 6 17 12 17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm		7	14	7	7	6	4	9	6	
17:00 11 14 7 7 15 4 23 20 17:15 7 14 6 6 20 6 17 12 17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	16:45	8	14	6	6	5	2	10	9	
17:30 9 12 7 8 17 6 15 12 17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	17:00	11	14	7	7	15	4	23	20	
17:45 8 14 8 6 8 4 14 11 18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	17:15	7	14	6	6		6	17	12	
18:00 7 14 7 8 14 4 15 11 18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	17:30	9	12	7	8	17	6	15	12	
18:15 9 14 7 7 17 5 11 7 Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	17:45	8	14	8	6	8	4	14	11	
Source: Matrix, Traffic & transport Data, 21/9/2023, 6am-10am and 3pm-7pm	18:00	7	14	7		14	4	15	11	
	18:15	9	14	7	7	17	5	11	7	
sed under the Go				7/0	ita, 21/9/2	2023, 6an	า-10am ar	nd 3pm-7	pm	

Table B2: Carlingford Road and Beecroft Road, Average Queue Length (Vehicles)

15-min Period			oft Road S)			В	eecroft Ro (N)	ad		Car	lingford R (W)	oad
Starting	Lane 1	Lane 2	Lane 3	Lane 4	Lane 1	Lane 2	Lane 3	Lane 4	Lane 5	Lane 1	Lane 2	Lane 3
6:30	1	1	9	8	8	9	9	2	1	1	7	7
6:45	1	1	8	7	7	7	9	2	1	1	8	8
7:00	1	1	8	8	8	10	12	2	2	1	8	7
7:15	1	1	12	12	9	9	11	4	3	0	8	8
7:30	1	1	13	12	9	10	11	3	3	0	8 6	9
7:45	1	1	10	10	8	9	9	4	3	0	8	8
8:00	1	1	20	20	7	8	11	5	4	0	7	8
8:15	1	1	14	15	8	9	11	5	3	10	9	8
8:30	2	1	12	13	7	8	10	6	5	0	9	8
8:45	1	1	9	10	7	9	8	5	4	0	9	8
16:00	3	4	14	17	4	4	4	6	3	1	9	9
16:15	2	3	22	25	4	3	2	5	4	1	9	9
16:30	3	3	20	23	4	3	3	6	5	0	9	9
16:45	4	3	19	24	5	4	5	7	5	0	9	8
17:00	4	4	16	21	4	3	4	9 7	5	0	9	9
17:15	3	3	17	21	5	4	4	6	5	0	9	9
17:30	4	4	18	24	7	5	8	8	6	1	9	9
17:45	4	4	15	19	4	40	6	8	4	1	9	9
18:00	2	3	13	16	4	4	3	5	3	0	9	9
18:15	2	2	12	15	5	5	4	4	3	1	9	8
3500	Inde		300	t Data, 2								

Table B3: High Street, Beecroft Road & Bridge Street, Average Queue Length (Vehicles)

	in Period arting	High Street (S)	Bridge Street (W)
	Ĭ	Lane 1	Lane 1
	6:30	0	1
	6:45	0	1
	7:00	1	2
	7:15	0	4
	7:30	1	4
	7:45	1	4
	8:00	1	3
	8:15	1	5
	8:30	1	4
	8:45	1	2
1	6:00	1	4
1	6:15	1	4
1	6:30	1	4
1	6:45	1	4
1	7:00	1	4
1	7:15	1	5
1	7:30	1	3
1	7:45	0	5
1	8:00	2	4
1	8:15	2	3
		(30 Netuly
20/02/50	Jung	in the	

Table B4: Rawson Street and Bridge Street, Average Queue Length (Vehicles)

	15-min Period	Rawson Street (S)	Bridge Street (E)	Rawson Street (N)	Bridge Street (W)	Right				
ļ	Starting				(**)					
		Lane 1	Lane 1	Lane 1	Lane 1					
	6:30	1	0	1	1					10
	6:45	1	1	1	1				X	
	7:00	2	2	1	2				70	
	7:15	2	2	6	3			-	\Y	
	7:30	2	2	4	2			C		
	7:45	3	2	5	3			25		
	8:00	2	3	4	5			()		
	8:15	4	7	4	4		0	5		
	8:30	3	2	2	3		~ Y~			
	8:45	2	4	3	3		\mathcal{O}			
	16:00	9	6	4	9	10,				
	16:15	3	4	3	5	00				
	16:30	7	8	7	8					
	16:45	2	6	3	2					
ļ	17:00	3	4	4	2					
	17:15	3	4	3	3					
	17:30	2	7	2	2					
	17:45	4	5	4(4					
	18:00	4	5	3	5					
I	18:15	2	4	2	3					
			l l			m-7pm				
	Source: Ma	atrix, Traffic & tra	ansport Data,			m-7pm				
	Source: Ma	atrix, Traffic & tra	ansport Data,			m-7pm				
	Source: Ma	atrix, Traffic & tra	ansport Data,			m-7pm				
	Source: Ma	atrix, Traffic & tra	ansport Data,			m-7pm				
	Source: Ma	atrix, Traffic & tra	ansport Data,			m-7pm				
	Source: Ma		ansport Data,			m-7pm				

Table B5: Epping Road, Blaxland Road and Langston Place, Average Queue Length (Vehicles)

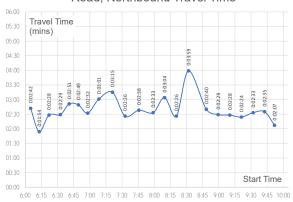
15-min Period	ВІ	axland Ro (S)	ad	E	pping Roa (E)	ıd	Langsto (N				ft Road V)	
Starting	Lane 1	Lane 2	Lane 3	Lane 1	Lane 2	Lane 3	Lane 1	Lane 2	Lane 1	Lane 2	Lane 3	Lane 4
6:30	3	2	4	1	4	3	1	2	2	6	5	11
6:45	5	1	2	1	11	8	1	3	1	4	5	6
7:00	4	3	3	0	12	7	2	3	0	11	5	13
7:15	5	2	5	1	20	14	3	5	1	16	11	31
7:30	7	4	4	1	16	11	2	4	1	23	22	28
7:45	8	5	5	1	20	13	5	6	6	23	23	27
8:00	6	3	7	1	20	15	4	8	12	10	10	15
8:15	7	5	7	1	22	16	3	7	4	10	9	14
8:30	8	5	5	1	14	11	4	4	5	12	12	26
8:45	7	3	5	1	10	8	2	4	7	1 20	17	20
16:00	37	36	4	1	31	26	3	4	4	5	4	25
16:15	36	36	4	1	24	17	3	4	4	6	6	27
16:30	23	20	4	1	26	21	3	4	4	4	4	28
16:45	37	34	4	2	25	22	5	5	1	6	7	32
17:00	40	37	3	1	26	20	3	O 5	3	4	3	26
17:15	43	43	4	2	25	20	4	6	5	4	5	31
17:30	41	38	5	2	29	23	4	6	8	7	9	28
17:45	22	20	5	2	25	23	4	6	5	9	7	33
18:00	15	11	3	3	23	15	2	5	7	4	5	31
18:15	12	7	4	1	14	7	2	4	4	7	7	13
3580	Matrix, 7		,	ein	Up							

Table B6: Epping Road and Essex Street, Average Queue Length (Vehicles)

15-min Period	Essex Street (S) Lane 1 Lane 2 Lane 3		E	Epping Road (E)	d	E	Essex Stree (N)	t	Epping (V	g Road V)	
Starting	Lane 1	Lane 2	Lane 3	Lane 1	Lane 2	Lane 3	Lane 1	Lane 2	Lane 3	Lane 1	Lane 2
6:30	1	1	2	1	3	3	1	3	3	12	12
6:45	1	1	1	0	3	2	2	4	4	13	12
7:00	1	1	3	1	2	2	1	5	6	12	13
7:15	1	1	3	1	3	3	2	9	10	14	13
7:30	1	1	3	1	2	1	2	6	8	14	14
7:45	2	1	4	1	3	2	1	9	8	14	13
8:00	1	2	5	1	3	2	2	9	9	12	13
8:15	1	2	4	1	4	3	2	9	8	13	12
8:30	1	3	6	1	3	3	3	7	7	10	13
8:45	1	3	5	1	4	4	3	7	J	11	12
16:00	1	2	4	2	31	28	2	10	9	12	11
16:15	2	2	4	1	30	27	4	6	8	12	12
16:30	3	1	3	1	31	27	4	7	7	12	12
16:45	2	2	5	1	30	29	2	8	7	12	12
17:00	1	3	4	1	32	27	3	12	13	11	12
17:15	1	4	6	0	31	27	2	6	6	11	11
17:30	2	5	7	1	30	28	5	10	11	12	12
17:45	1	3	5	1	29	24	4	10	8	11	11
18:00	1	2	4	1	22	18	2	11	11	10	9
18:15	1	2	4	1	8	7	2	6	5	6	6
Source:			10	KULU	5 `						

C. Travel Time Data (21/9/2023)

Beecroft Road, Epping Road and Blaxland Road, Northbound Travel Time



Beecroft Road, Epping Road and Blaxland Road, Southbound Travel Time



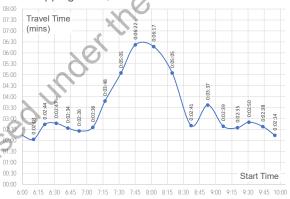
Beecroft Road, Epping Road and Blaxland Road, Northbound Travel Time



Beecroft Road, Epping Road and Blaxland Road, Southbound Travel Time



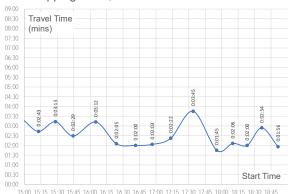
Carlingford Road, Beecroft Road and Epping Road, Eastbound Travel Time



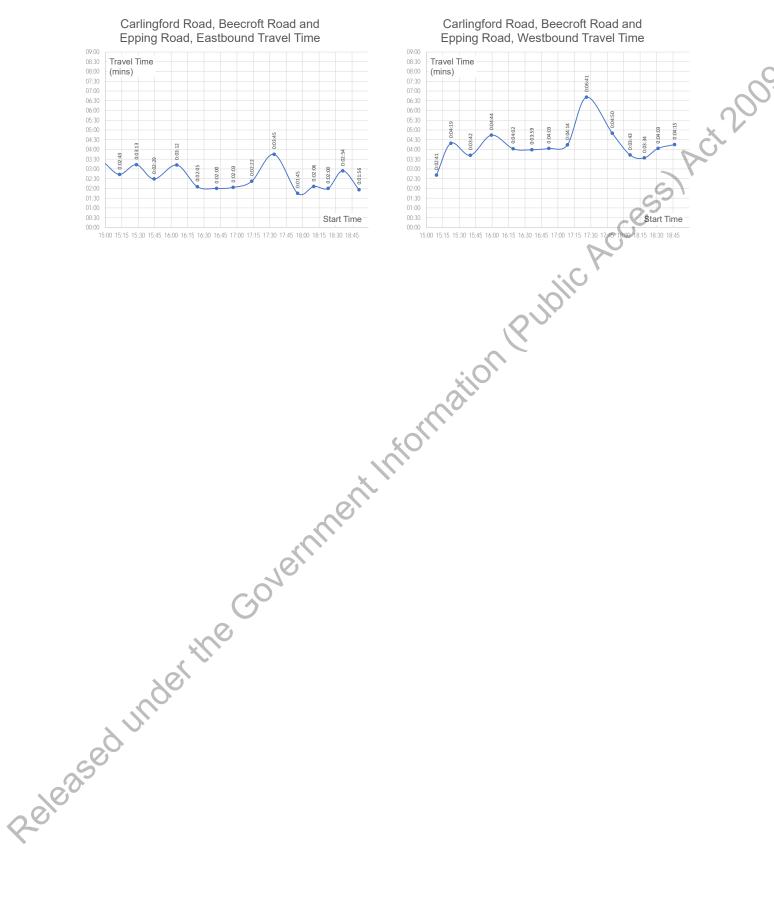
Carlingford Road, Beecroft Road and Epping Road, Westbound Travel Time



Carlingford Road, Beecroft Road and Epping Road, Eastbound Travel Time



Carlingford Road, Beecroft Road and Epping Road, Westbound Travel Time



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D. Site Visit Photographs

Beecroft Road NB at Carlingford Rd / Beecroft Road Intersection



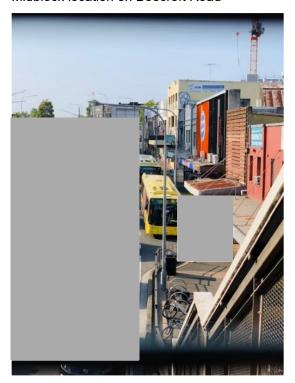
Beecroft Road NB at Carlingford Rd / Beecroft Road Intersection

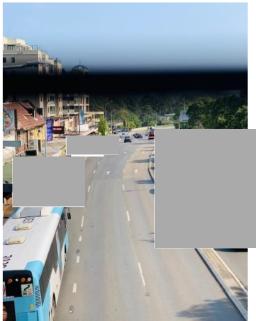


Bus Stop Stand on Beecroft Road



Midblock location on Beecroft Road





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E. SCATS Traffic Count Data (21/9/2023)

Table E1: TCS216, AM Peak, 15-min Detector Counts (Vehicles)

15-min Period Starting	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	Total
6:30	3	98	73	204	248	56	26	115	22	8	15	868
6:45	2	113	82	186	232	62	25	129	20	7		875
7:00	1	125	101	208	249	65	32	121	26	13	24	965
7:15	5	145	137	207	277	76	40	139	32	19	39	1116
7:30	4	148	143	217	286	94	51	133	36	12	36	1160
7:45	4	149	123	202	276	102	64	136	40	17	45	1158
8:00	4	137	140	198	274	97	51	134	55	23	56	1169
8:15	3	152	156	205	264	97	55	120	50	16	52	1170
8:30	10	129	124	191	235	78	44	125	38	19	39	1032
8:45	6	134	110	190	256	111	48	132	29	14	32	1062

Table E2: TCS216, PM Peak, 15-min Detector Counts (Vehicles)

						4						
15-min Period Starting	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	Total
17:00	8	206	211	119	147	103	95	65	39	21	32	1046
17:15	13	209	215	139	164	104	118	70	41	20	32	1125
17:30	9	203	215	132	157	109	115	61	38	18	33	1090
17:45	10	214	204	140	175	101	109	67	33	22	46	1121
18:00	8	231	221	107	151	101	111	62	30	20	38	1080
18:15	10	208	214	131	170	94	108	60	33	24	48	1100
18:30	12	199	204	170	196	100	106	65	31	28	37	1148
18:45	13	205	204	157	187	103	102	61	44	19	44	1139
19:00	17	219	214	149	150	95	80	58	35	17	34	1068
19:15	3	198	179	136	147	106	84	78	37	12	23	1003

Table E3: TCS706, AM Peak, 15-min Detector Counts (Vehicles)

15-min Period Starting	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	D12	TOTAL
6:30	62	75	76	72	85	45	58	14	3	167	208	23	888
6:45	67	68	62	76	98	56	80	8	8	157	185	24	889
7:00	64	89	100	94	110	60	81	11	9	159	199	30	1006
7:15	86	107	108	104	111	76	97	25	17	167	204	21	1123
7:30	100	123	100	122	136	86	127	23	17	152	179	10	1175
7:45	102	118	89	96	113	84	116	21	16	163	191	12	1121
8:00	118	147	101	111	101	72	74	29	21	142	179	12	1107
8:15	121	141	93	118	102	84	94	27	19	159	180	19	1157
8:30	96	121	92	120	113	65	98	41	32	159	176	10	1123
8:45	94	101	88	96	93	55	84	36	20	159	166	11	1003

Table E4: TCS706, PM Peak, 15-min Detector Counts (Vehicles)

	15-min Period Starting	D1	D2	D3	D4	D5	D6	Ð7	D8	D9	D10	D11	D12	TOTAL
	17:00	124	168	53	63	62	112	124	36	22	115	125	25	1029
	17:15	158	193	56	77	67	110	109	36	25	115	124	27	1097
	17:30	152	183	52	79	65	119	119	37	40	116	138	15	1115
	17:45	161	183	73	73	59	104	113	36	31	117	127	20	1097
	18:00	152	187	61	64	52	120	120	39	34	102	122	30	1083
	18:15	155	186	74	80	64	104	117	41	36	108	125	20	1110
	18:30	140	172	96	122	49	85	123	48	45	120	127	23	1150
	18:45	148	183	72	91	57	106	122	40	31	132	138	11	1131
	19:00	139	179	64	69	45	107	114	30	23	113	133	15	1031
	19:15	116	149	60	65	50	107	112	32	18	124	132	11	976
Relea	58911													

Table E5: TCS1015, AM Peak, 15-min Detector Counts (Vehicles)

15-min Period Starting	D1	D2	D3	D4	D5	D6	D7	D8	Total
6:30	170	186	56	62	22	10	5	11	522
6:45	151	167	70	84	27	17	12	18	546
7:00	162	178	69	85	32	21	19	19	585
7:15	163	169	90	113	44	39	31	18	667
7:30	125	139	114	136	65	44	32	17	672
7:45	122	144	103	125	60	55	45	15	669
8:00	114	134	94	101	64	48	45	19	619
8:15	122	136	106	114	62	51	43	17	651
8:30	117	130	101	124	52	41	43	13	621
8:45	123	123	88	107	62	48	39	20	610

Table E6: TCS1015, PM Peak, 15-min Detector Counts (Vehicles)

_										
	15-min Period Starting	D1	D2	D3	D4	O _{D5}	D6	D7	D8	Total
	16:00	118	108	144	150	21	32	61	7	641
	16:15	116	101	153	128	16	24	55	10	603
	16:30	121	105	153	148	23	31	48	6	635
	16:45	117	109	131	146	28	14	55	14	614
	17:00	114	98	157	157	7	23	77	10	643
	17:15	125	107	136	153	8	43	59	7	638
	17:30	114	111	147	149	15	35	71	8	650
	17:45	136	109	145	141	31	37	57	6	662
	18:00	119	113	136	128	7	28	62	11	604
	18:15	122	114	129	126	27	35	50	8	611
20/08	sedvi	uge.								

Table E7: TCS1338, AM Peak, 15-min Detector Counts (Vehicles)

15-min Period Starting	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	Total
6:30	193	231	4	72	53	8	19	19	2	4	12	617
6:45	202	205	1	81	52	9	24	25	2	2	8	611
7:00	212	224	3	83	54	10	36	49	4	8	15	698
7:15	218	234	7	92	88	22	52	59	4	8	24	808
7:30	242	248	5	97	86	26	40	50	3	6	22	825
7:45	221	232	6	97	68	15	52	54	7	10	31	793
8:00	222	234	5	91	73	20	50	57	8	13	36	809
8:15	229	227	8	97	78	24	58	63	6	12	28	830
8:30	206	198	6	87	72	28	35	45	6	14	48	745
8:45	217	211	12	69	61	24	39	46	4	16	39	738

Table E8: TCS1338, PM Peak, 15-min Detector Counts (Vehicles)

	15-min Period Starting	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	Total
	16:00	124	103	13	130	159	26	47	58	2	13	27	702
	16:15	131	132	13	146	180	29	40	52	9	16	28	776
	16:30	120	122	18	149	175	29	37	47	16	8	27	748
	16:45	134	138	9	133	159	20	48	48	12	15	32	748
	17:00	127	105	10	134	163	31	58	79	9	18	30	764
	17:15	132	116	12	138	172	22	33	43	11	20	41	740
	17:30	148	158	9	109	142	35	47	72	10	33	42	805
	17:45	153	147	9	136	146	35	43	52	4	19	39	783
	18:00	131	114	16	139	157	28	55	60	2	15	31	748
	18:15	136	106	10	147	148	22	36	40	5	14	29	693
2-6/63	sed	JIN OF											
i													

F. SCATS Intersection and Subsystem Data

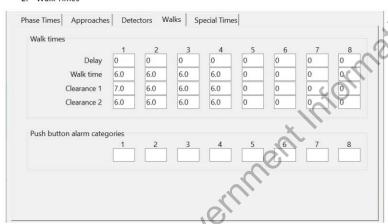


TCS 1015

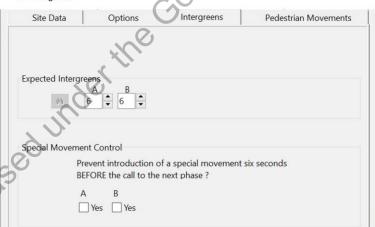
1. Phase Times

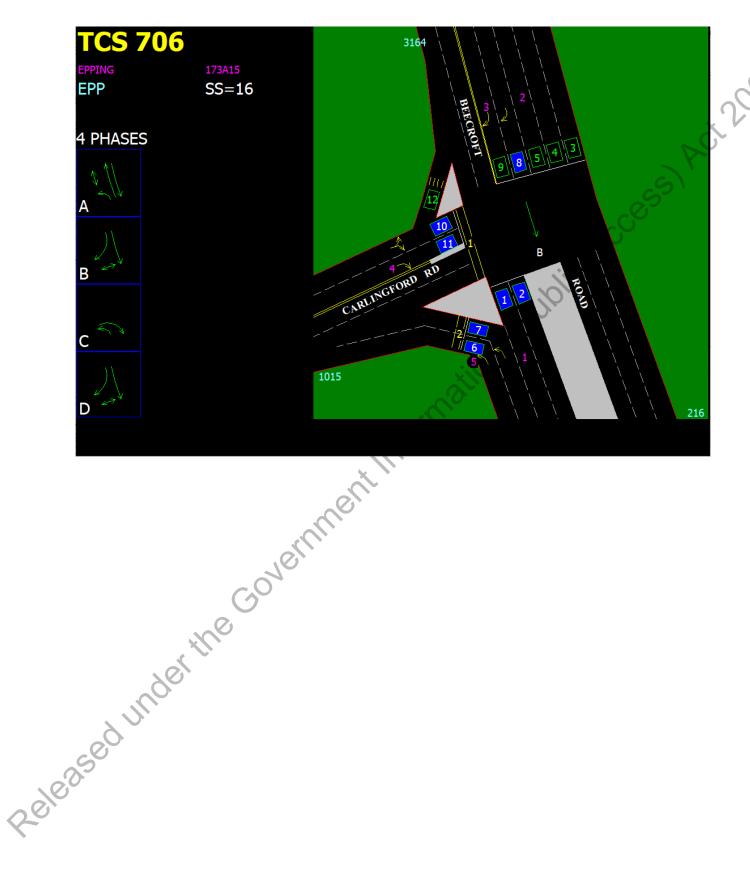
TCS 1015	
1. Phase Times	
Phase Times Approaches Detectors Walks Special Times	Rublic Access Act. 2003
2. Walk Times Phase Times Approaches Detectors Walks Special Times Walk times 1 2 3 4 5 6 7 8 Delay 0 0 0 0 0 0 0 0 0 Walk time 6.0 6.0 6.0 6.0 6.0 0 0 0 0 Clearance 1 7.0 6.0 6.0 6.0 0 0 0 0 0 0	ijon (Public

2. Walk Times



3. Intergreens





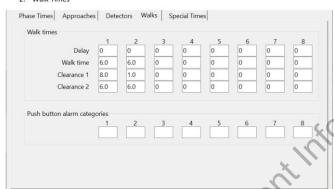
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TCS 706

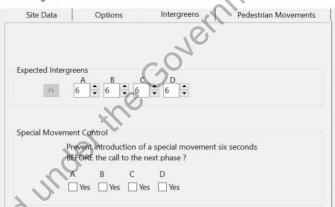
1. Phase Times



2. Walk Times



3. Intergreens





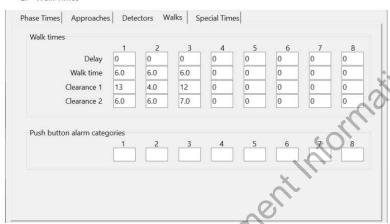
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TCS 216

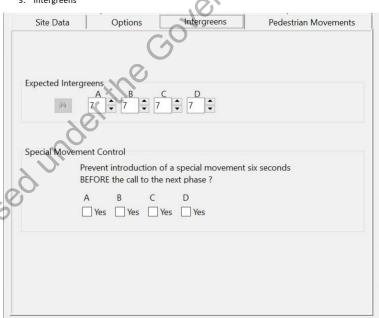
1. Phase Times

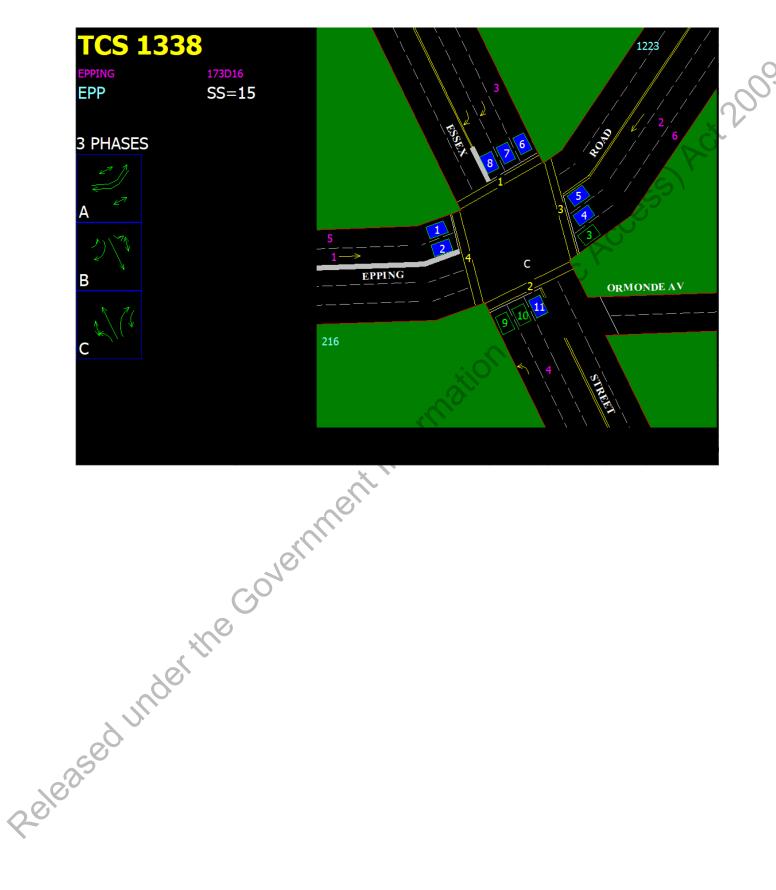


2. Walk Times



3. Intergreens





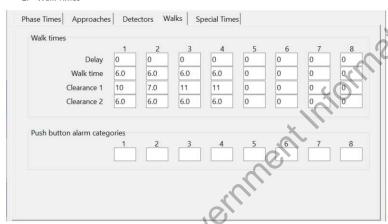
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TCS 1338

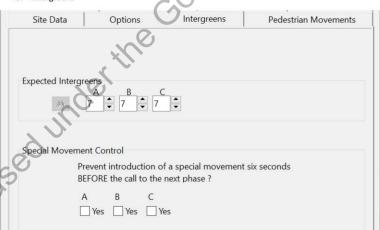
1. Phase Times



2. Walk Times



3. Intergreens



G. SCATS Region LX File

1338
SLOT15=3,1,4!INT=1338IVC=6.1ICS=16!PK=/ZSL=0! [Controller and software Information – No subsystem data is assigned to this site]
COM=DD,P,1:4,H!BPS=1200!PH=0298686232!DD=0!CTYPE=C29V6.1R20S18!
LS=ON!
IK=!
S#=15!LM=MF^IRMN=0!DCL=0!
VOLS=1-24!
AT=7!BT=7!CT=7!
W1=6!W1T=16!W1F=!W2=6!W2T=13!W2F=! [Pedestrian Walk and Clearance]
W3=6!W3T=17!W3F=!W4+6!W4T=17!W4F=!
PP1=22,274)PP2==3,13**|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P11=21*|V4F=14|P

SLOT18=2,1,4!INT=1015!VC=6.1!CS=354!PK=/ZSL=0! [Controller and Software Information] COM=DO,P,3:2,H!BPS=1200!PH=0298770813!DD=0!CTYPE=C29V6.1R20S18!

IK=!

S#=16!LM=MF^!RMN=0!DCL=0! [Subsystem 16 assigned to this site]

VOLS=1-24!

AT=6!BT=6! [Phase intergreen]

W1=8!W1T=13!W1F=!W2=8!W2T=12!W2F=! [Pedestrian Walk and Clearance]

W3=6!W3T=12!W3F=!W4=6!W4T=12!W4F=!

PP1=-7,-7A!PP2=-7,-7A! [Offset Plan Data]

PP3=-7,-7A!PP4=-7,-7A!

VAR1=47!VAR1.0=!

VAR1.1=2!VAR1.2=142!VAR1.3=30!VAR1.4=30!

VAR2=75IVAR2 0=1

VAR2.1=1283!

I=1015!PLAN=1!SF=!XSF=0! [Split plan data]

A=0PDFGB!

B=35A!

SS=16!LCL=40!HCL=130!SCL=72,0!KCL=0!ZSS=0! [Subsystem Data assigned to Site 1015, with Min / Max Cycle Length = 40 seconds / 130 seconds. Stopper 1 = 72 seconds]

SK=NSDDNAOVIFBF!

XCL=110!SZ=88,94!SMX=/ [Stretch Cycle Time and its cycle length calibration factors]

FCL=30,40,45,50,60,70,75,80,90,100,110,115,120,125,130,140!

PS1=40^,72!PS2=82,110!PS3=82,110!PS4=82,110!

LP1=-24,-24^D216! [Link Plan Data - Site 1015 is linked to Site 216]

LP2=30,16^A216! LP3=9,3^D216!

LP4=8.39^D216!

706

```
Regovernment Information Public Access Act 2009
 SLOT17=4.4.2!INT=706!VC=5!CS=21!PK=/ZSL=0!
 COM=NET,H!CTYPE=C18V5R20S24!
 LS=ON!
 IK=!
 S#=16!LM=MF^!RMN=0!DCL=0! [Subsystem assigned to this site.
 VOLS=1-24!
 AT=6!BT=6!CT=6!DT=6! [Phase intergreen]
 W1=6!W1T=14!W1F=!W2=6!W2T=7!W2F=! [Pedestrian Walk and Clearance]
 PP1=0,0^B!PP2=0,0^B! [Offset Plan data]
 PP3=0,0^B!PP4=0,0^B!
 VAR1=35!VAR1.0=!
 VAR1.1=1!VAR1.2=3!VAR1.3=0!VAR1.4=0!VAR1.5=0!
 VAR1.6=0!VAR1.7=0!VAR1.8=0!VAR1.9=37!VAR1.10=147!
 VAR1.11=0!VAR1.12=0!VAR1.13=0!
 VAR2=11!VAR2.0=!
 VAR2.1=127!VAR2.2=10!VAR2.3=0!VAR2.4=19!VAR2.5=0!
 VAR3=3!VAR3.0=!
 VAR3.1=0!VAR3.2=1!
 VAR4=49!VAR4.0=!
 VAR4.1=65!VAR4.2=0!
 VAR5=10!VAR5.0=!
 VAR6=11!VAR6.0=!
 VAR6.1=383!VAR6.2=7!VAR6.3=0!VAR6.4=19!VAR6.5=0!
 VAR7=90!VAR7.0=!
 VAR7.1=1!
 VAR8=10!VAR8.0=!
 VAR9=75!VAR9.0=!
 VAR9.1=1295!
 I=706!PLAN=1!SF=!XSF=0! [Split plan data]
 A=49TGC!
 C=0PDFGB!
 B=15FGA!
 D=1A!
 I=706!PLAN=2!SF=!XSF=0!
 A=45TGC!
 C=0PDFGB!
 B=15FGA!
 D=1A!
 I=706!PLAN=3!SF=!XSF=0!
 A=41TGC!
 C=0PDFGB!
 B=15FGA!
 D=1A!
 I=706!PLAN=4!SF=!XSF=0!
 A=37TGC!
 C=0PDFGB!
 B=15FGA!
D=1A!
```

SS=16!LCL=40!HCL=130!SCL=72,0!KCL=0!ZSS=0! [Subsystem Data assigned to Site 706, with Min / Max Cycle Length = 40 seconds / 130 seconds. Stopper 1 = 72 seconds1 SK=NSDDNAOVIFBF! XCL=110!SZ=88,94!SMX=/ [Stretch Cycle Time and its cycle length calibration factors]

FCL=30,40,45,50,60,70,75,80,90,100,110,115,120,125,130,140!

PS1=40^,72!PS2=82,110!PS3=82,110!PS4=82,110!

LP1=-24,-24^D216! [Link Plan Data Site 706 is linked to Site 216]

LP2=30,16^A216!

LP3=9,3^D216! LP4=8,39^D216!

I=216!PLAN=5!SF=Z+!XSF=0!

A=0PDFGB!

216

```
Information (Public Access) Act 2009
 SLOT16=4,8,3!INT=216!VC=6.1!CS=345!PK=/ZSL=0! [Controller and Software Information]
 COM=NET,H!CTYPE=C29V6.1R20S18!
 LS=ON!
 IK=!
 S#=15!LM=MI^!RMN=0!DCL=80! [Subsystem 15 Assigned to this Site]
 VOLS=1-24!
 AT=7!BT=7!CT=7!DT=7! [Phase intergreen]
 W1=6!W1T=19!W1F=!W2=0ABD!W2T=10!W2F=! [Pedestrian Walk and clearance]
 W3=6!W3T=19!W3F=!
 PP1=0,0^D!PP2=0,0^A! [Offset plan data]
 PP3=0,0^D!PP4=0,0^D!
 VAR1=35!VAR1.0=!
 VAR1.1=1!VAR1.2=2!VAR1.3=3!VAR1.4=0!VAR1.5=0!
 VAR1.6=4!VAR1.7=0!VAR1.8=0!VAR1.9=158!VAR1.10=19!
 VAR1.11=145!VAR1.12=0!VAR1.13=107!
 VAR2=14!VAR2.0=!
 VAR2.1=0!VAR2.2=15!
 VAR3=14!VAR3.0=!
 VAR3.1=0!VAR3.2=7!
 VAR4=19!VAR4.0=!
 VAR5=14!VAR5.0=!
 VAR5.1=0!VAR5.2=3!
 VAR6=36!VAR6.0=!
 VAR6.1=300!VAR6.2=301!
 VAR7=10!VAR7.0=!
VAR8=11!VAR8.0=!
VAR8.1=383!VAR8.2=7!VAR8.3=0!VAR8.4=19!VAR8.5=0!
VAR9=90!VAR9.0=!
VAR9.1=2!
VAR10=75!VAR10.0=!
VAR10.1=1311!

I=216!PLAN=1!SF=!XSF=0! [Split plan data]
A=0PDFGB!
B=19FGC!
C=17A!
D=1A!
 VAR8=11!VAR8.0=!
 D=1A!
 I=216!PLAN=2!SF=!XSF=0!
 A=0PDFGB!
 B=22FGC!
 C=17A!
 D=1A!
 I=216!PLAN=3!SF=!XSF=0!
 A=0PDFGB!
 B=25FGC!
  C=17A!
D=1A!
  I=216!PLAN=4!SF=!XSF=0!
  A=0PDFGB!
 B=28FGC!
 C=17A!
 D=1A!
```

B=14FGC! C=17D! D=17TGA!

JF=0!

40IHCL=130ISCL=72,0IKCL=0IZSS=0! [Subsystem Data assigned to Si. ads. Stopper 1 = 72 seconds]

JDNA0VIEI

1.0ISZ=88,94ISM2 | [Stretch Cycle Time and its cycle length calibration fabtors]

-30.40.45,50.80,70.75,80.90.100.110.115,120.125,130,140!

31=40,72IPSZ=82,110IPS3=82,110IPS4=82,110!

LP1=0! [Link Plan Data]

LP2=0!

LP3=0!

LP4=0! JANIC ACCESS ACT 2009
The Mir SS=15!LCL=40!HCL=130!SCL=72,0!KCL=0!ZSS=0! [Subsystem Data assigned to Site 216, with Min / Max Cycle Length = 40 seconds

H. SCATS History Files

3 - Less Act 2009 Periodic statistics for TCS sites 216, 706, 1015 and 1338, at 15-min intervals from Thursday, 21 September 2023, 12:00 AM AEST to Thursday, 21 September 2023, 11:59:59 PM AEST.

TCS 216		Observe	d Averag	je Phase	Times (s	secs) for	the 15-n	nin perio	d starting	J
103 210	6:30	6:45	7:00	7:15	7:30	7:45	8:00	8:15	8:30	8:45
A-phase	55	47	55	53	55	54	52	58	55	53
B-phase	31	35	27	26	23	23	23	23	23	26
C-phase	24	24	24	25	26	26	27	26	26	26
D-phase	20	22	23	23	24	25	25	22	24	24
Cycle Length	129	125	127	130	130	130	130	132	130	130

TCS 216		Observe	d Averag	je Phase	Times (secs) for	the 15-n	nin perio	d starting	J
103 210	16:00	16:15	16:30	16:45	17:00	17:15	17:30	17:45	18:00	18:15
A-phase	81	81	81	80	81	80	81	81	81	73
B-phase	23	23	23	23	23	23	23	23	23	30
C-phase	25	25	26	26	25	25	25	26	25	25
D-phase	-	-	1	-	-)	-	-	ı
Cycle Length	129	129	130	130	129	130	130	130	130	129

TCS 706		Observe	d Averag	e Phase	Times (secs) for	the 15-n	nin perio	d starting	
103700	6:30	6:45	7:00	7:15	7:30	7:45	8:00	8:15	8:30	8:45
A-phase	32	32	37	41	48	46	47	47	43	43
B-phase	14	15	15	16	17	18	18	17	19	19
C-phase	82	77	78	70	63	65	63	63	66	67
Cycle Length	130	124	129	130	129	127	129	127	128	129
	۵C	Ø								
		Obconio	d Avoros	o Dhaca	Timos (oooo) for	tho 15 n	ain norio	d atartina	

TCS 706	3	Observe	d Averag	e Phase	Times (s	secs) for	the 15-n	nin perio	d starting	I
103 700	16:00	16:15	16:30	16:45	17:00	17:15	17:30	17:45	18:00	18:15
A-phase	60	60	57	57	58	59	56	56	59	52
B-phase	19	19	19	19	20	19	20	19	18	19
C-phase	50	50	52	52	50	49	54	54	52	58
Cycle Length	129	130	129	132	129	128	130	130	130	128

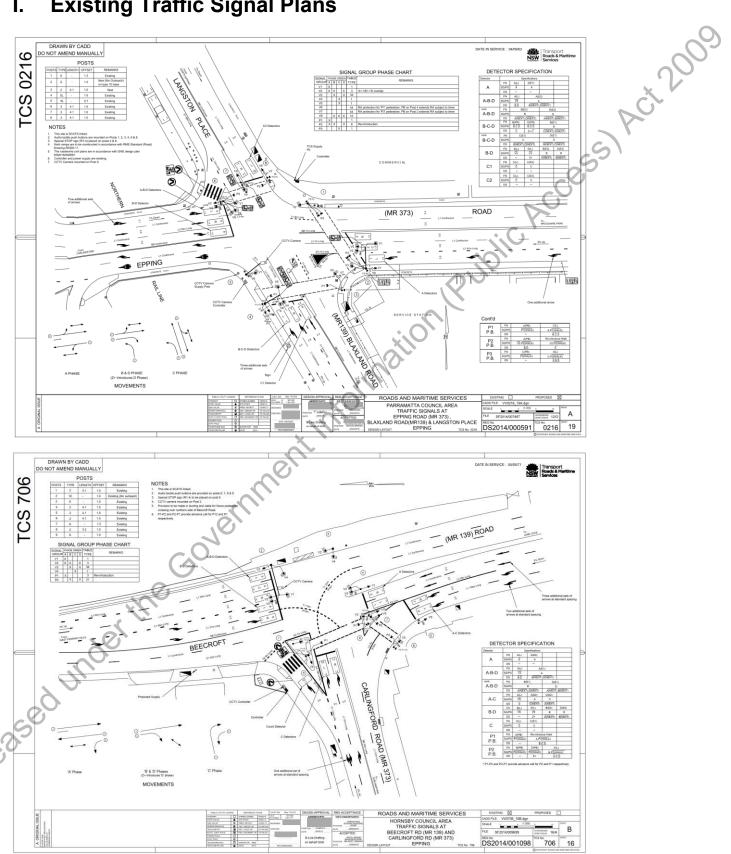
TCS 1015		Observe	d Averag	e Phase	Times (s	secs) for	the 15-n	nin perio	d starting	
103 1013	6:30	6:45	7:00	7:15	7:30	7:45	8:00	8:15	8:30	8:45
A-phase	104	96	100	91	87	86	85	84	84	84
B-phase	24	28	28	37	43	43	44	45	46	46
Cycle Length	129	124	132	128	131	129	129	130	130	130

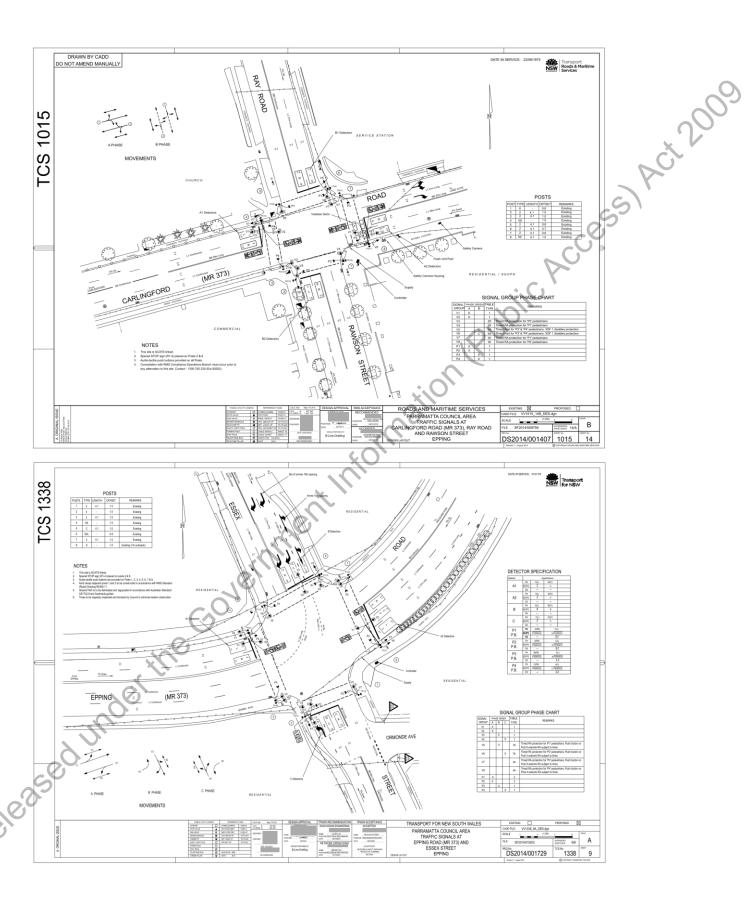
TCS 1015		Observe	d Averag	e Phase	Times (secs) for	the 15-n	nin perio	d starting		
103 1015	6:30	6:45	7:00	7:15	7:30	7:45	8:00	8:15	8:30	8:45	
A-phase	104	96	100	91	87	86	85	84	84	84	
B-phase	24	28	28	37	43	43	44	45	46	46	
Cycle Length	129	124	132	128	131	129	129	130	130	130	72
											ACT 20
TCS 1015	(Observe	d Averag	e Phase	Times (secs) for	the 15-n	nin perio	d starting		
103 1013	16:00	16:15	16:30	16:45	17:00	17:15	17:30	17:45	18:00	18:15	355
A-phase	84	84	84	84	84	84	84	85	84	84	25
B-phase	46	46	46	46	46	46	46	45	46	46	
Cycle Length	130	130	130	130	130	130	130	130	130	130	
										1	-
									(1)	<u>ن</u>	
									101	•	
	l								\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\]
TCS 1338		Observe			,						
	6:30	6:45	7:00	7:15	7:30	7:45	8:00	8:15	8:30	8:45	
A-phase	95	94	85	81	87	80	81	79	80	80	

TOO 4000		Observe	d Averac	e Phase	Times (secs) for	the 15-n	nin perio	d starting	ı
TCS 1338	6:30	6:45	7:00	7:15	7:30	7:45	8:00	8:15	8:30	8:45
A-phase	95	94	85	81	87	80	81	79	80	80
B-phase	18	19	25	28	26	27	26	29	25	26
C-phase	16	15	18	21	19	22	22	21	24	22
Cycle Length	130	127	129	131	129	130	130	129	130	130

	TCS 1338		Observe	d Averag	e Phase	Times (secs) for	the 15-n	nin perio	d starting	l
	103 1336	16:00	16:15	16:30	16:45	17:00	17:15	17:30	17:45	18:00	18:15
	A-phase	81	81	82	79	78	81	78	79	80	83
	B-phase	26	26	25	26	29	24	27	26	27	26
	C-phase	22	20	20	22	21	23	24	23	21	21
	Cycle Length	130	127	128	129	129	131	130	130	130	130
200	Cycle Length	es in	S								

Existing Traffic Signal Plans I.





Mott MacDonald Page **65** of **69**

J. 2023 Bus Service Coding Summary

Table J1: AM Peak Bus Services

Γable	J1: <i>A</i>	AM Pe	eak Bu	ıs Ser	vices											,55) ACT 2009
	St	art	Eve	ent 1	Eve	nt 2	Eve	nt 3	Eve	nt 4	Eve	nt 5	Eve	nt 6	End	
Route	Start	Cordon	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Cordon	~0"
	Time	Entry	Dus Stop	(sec)	bus stop	(sec)	bus Stop	(sec)	bus stob	(sec)	Dus Sioh	(sec)	Dus Sioh	(sec)	Exit	
541	06:47	1	212112	300	212114	300	212190	10	212115	10	-	-	0	0	1	
541 (EB)	07:35	1	212112	300	212114	-	-	-	-	-	-	-	0	0	1	-61
541	08:02	1	212112	240	212114	300	212190	10	212115	10	-	-	0	0	1 (5
541	08:54	1	212112	300	212114	300	212190	10	212115	10	-	-	0	0		,
541 (WB)	07:28	3	212114	300	212190	10	212115	10	- 010110	- 200	- 212140	300	0 2121163	0	10	
546 546	06:37 07:05	3	2121155 2121155	10 10	212190 212190	10	212115 212115	10 10	212112 212112	300 300	212140 212140	300	2121163	10 10	3	
546	07:35	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3	
546	08:21	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	(10)	3	
546	08:54	3	2121155		212190	10	212115	10	212112	300	212140	300	2121163	10	3	
549 (NB)	06:44	3	212140	300	2121163	10	-	-		-	-	-	XO		3	
549	06:52	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3	
49 (NB)	07:14	3	212140	300	2121163	10	-	-	-	-	-	10		-	3	
549	07:20	3	2121155		212190	10	212115	10	212112	300	212140	300	2121163	10	3	
549	07:49	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3	
549	08:05	3	2121155		212190	10	212115	10	212112	300	212140	300	2121163	10	3	
549	08:38	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3	
549 (SB)	08:58	3	2121155	10	212190	10	212115	10	212112	300	17,	-	0	0	3	
550 (EB)	06:54	2	2121213	10	212111	300	2121214	10	-	مزر	Γ.	-	0	0	7	
50 (EB)	07:03	2	2121213	10	212111	300	2121214	10	-	() ,	-	-	0	0	7	
550 (EB) 550 (EB)	07:15 07:25	2	2121213 2121213	10 10	212111 212111	300 300	2121214 2121214	10 10		-	-	-	0	0	7	
550 (EB)	07:25	2	2121213	10	212111	300	2121214	10	XU	-	-	-	0	0	7	
550 (EB)	07:57	2	2121213	10	212111	300	2121214	10	1	-	-	-	0	0	7	
550 (EB)	08:10	2	2121213	10	212111	300	2121214	10	-	-	-	-	0	0	7	
550 (EB)	08:20	2	2121213	10	212111	300	2121214	10	-	-	-	-	0	0	7	
550 (EB)	08:31	2	2121213	10	212111	300	2121214	10	-	-	-	-	0	0	7	
550 (EB)	08:41	2	2121213	10	212111	300	2121214	10	-	-	-	-	0	0	7	
550 (EB)	08:53	2	2121213	10	212111	300	2121214	10	-	-	-	-	0	0	7	
50 (WB)	06:35	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB)	06:46	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB)	06:56	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB)	07:06	7	212138	10	212126 212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB) 50 (WB)	07:14 07:25	7	212138 212138	10	212126	10	212154 212154	300 300	212190 212190	10	-	-	0	0	1	
550 (WB)	07:37	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
550 (WB)		7	212138	10	212126	10	212154	300	212190	10	_	-	0	0	1	
50 (WB)	08:01	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB)	08:11	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB)	08:22	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
50 (WB)	08:44	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
550 (WB)	08:53	7	212138	10	212126	10	212154	300	212190	10	-	-	0	0	1	
30 (WB)	06:32	2	212114	300	212129	10	-	-	-	-	-	-	0	0	2	
630	07:20	2	212123	10	212190	10	212115	10	212114	300	212129	10	-	-	2	
630	07:50	2	212123	10	212190	10	212115	10	212114	300	212129	10	-	-	2	
630 51 (NB)	08:36	2	212123 212113	10 300	212190	10	212115	10	212114	540	212129	10	- 0	- 0	2	
651 (NB)	06:30 06:47	4	212113	10	212115	10	212113	300	-	-	-	-	0	0	4	
651	07:10	4	212190	10	212115	10	212113	300	-	-	-	-	0	0	4	
651	07:35	4	212170	10	212115	10	212113	300	-	-	-	-	0	0	4	
651	08:03	4	212170	10	212115	10	212113	300	-	-	-	-	0	0	4	
51 (SB)	08:25	4	212190	10	212115	10	212113	300	0	0	0	0	0	0	4	
651	08:50	4	212190	10	212115	10	212113	300	-	-	-	-	0	0	4	
718w	08:25	1	212114	300	212190	10	212115	10	-	-	-	-1	0	0	1	
726w	07:33	2	212114	300	212129	10	-	-	-	-	-	-	0	0	2	
732w	07:50	2	212114	300	212129	10	-	-	-	-	-	-	0	0	2	
3013	08:02	2	212114	300	-	-	-	-	-	-	-	-	0	0	2	
3014	07:57	2	212114	300	-	-	-	-	-	-	-	_	0	0	2	

Table J2: PM Peak Bus Services

	St	art	Eve	nt 1	Eve	nt 2	Event	:3	Eve	ent 4	Eve	ent 5	Event 6		End
Route	Start	Cordon	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Bus Stop	Dwell	Cordon
F.14	Time	Entry	040440	(sec)	040444	(sec)	040400	(sec)	040445	(sec)	'	(sec)	,	(sec)	Exit
541	16:22	1	212112	300	212114	300	212190	10	212115	10	-	-	0	0	1
541	17:07	1	212112	300	212114	-	-	-	- 040445	-	-	-	0	0	1
541	17:37	1	212112	300	212114	300	212190	10	212115	10	-	-	0	0	1
541	18:24	1	212112	300	212114	300	212190	10	212115	10	- 212140	- 200	0	0	1
546	16:23	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3
546	16:53	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3
546 546	17:23 17:53	3	2121155 2121155	10 10	212190 212190	10	212115 212115	10 10	212112 212112	300 300	212140 212140	300 300	2121163 2121163	10 10	9 3
546(SB)	18:23	3	2121155	10	212190	10	212115	10	212112	300	212140	300	0	0	3
549	16:08	3		10		10		10	212112	300			2121163	10	3
			2121155		212190	_	212115				212140	300		10	
549 549	16:38 17:08	3	2121155 2121155	10 10	212190 212190	10	212115 212115	10 10	212112 212112	300 300	212140 212140	300 300	2121163 2121163	10	3
549	17:08	3	2121155	10	212190	10	212115	10	212112	300	212140	300	2121163	10	3
549 (NB)	18:00	3	2121155	300	212190	10	212113	- 10	212112	300	212140	300	2121103	- 10	3
549 (NB)	18:08	3	212140	10	2121103	10	212115	10	212112	120	212140	300	2121163	10	3
550 (EB)	16:17	2	2121133	10	212190	300	212115	10	- 212112	-	212140	300	0	0	7
550 (EB)	16:26	2	2121213	10	212111	300	2121214	10	-	-		1	0	0	7
550 (EB)	16:36	2	2121213	10	212111	300	2121214	10	-	_	V		0	0	7
550 (EB)	16:46	2	2121213	10	212111	300	2121214	10	_	_	1.		0	0	7
550 (EB)	16:56	2	2121213	10	212111	300	2121214	10	-	-	1		0	0	7
550 (EB)	17:16	2	2121213	10	212111	300	2121214	10				-	0	0	7
550 (EB)	17:37	2	2121213	10	212111	300	2121214	10					0	0	7
550 (EB)	17:57	2	2121213	10	212111	300	2121214	10	- 0			_	0	0	7
550 (EB)	18:17	2	2121213	10	212111	300	2121214	10			-		0	0	7
550 (LB)	16:00	7	212138	10	212111	10	212154	300	212190	10			0	0	1
550 (WB)	16:24	7	212138	10	212126	10	212154	300	212170	10		_	0	0	1
550 (WB)	16:36	7	212138	10	212126	10	212154	300	212190	10			0	0	1
550 (WB)	16:46	7	212138	10	212126	10	212154	300	212190	10			0	0	1
550 (WB)	16:56	7	212138	10	212126	10	212154	300	212170	10	_	_	0	0	1
550 (WB)	17:16	7	212138	10	212126	10	212154	300	212170	10	_	_	0	0	1
550 (WB)	17:37	7	212138	10	212126	10	212154	300	212170	10	_	_	0	0	1
550 (WB)	17:57	7	212138	10	212126	10	212154	300	212190	10			0	0	1
550 (WB)	18:17	7	212138	10	212126	10	212154	300	212190	10	_	_	0	0	1
630 (EB)	16:25	2	212123	10	212190	10	212134	10	212114	300	_	_	0	0	2
630 (EB)	16:55	2	212123	10	212190	10	212115	10	212114	300	_	_	0	0	2
630	17:25	2	212123	10	212170	10	212115	10	212114	300	212129	10	-	-	2
630	17:55	2	212123	10	212190	10	212115	10	212114	300	212129	10	-	-	2
630	18:25	2	212123	10	212190	10	212115	10	212114	300	212129	10	-	-	2
651 (NB)	16:00	4	212113	300	-	-	-	-	-	-		-	0	0	4
651	16:17	4	212190	10	212115	10	212113	300	-	-	-	-	0	0	4
651	16:43	4	212190	10	212115	10	212113	300	_	-	-	_	0	0	4
651	17:12	4	212190	10	212115	10	212113	300	_	-	-	_	0	0	4
651 (NB)	17:40	4	212113	300	-	-	-	-	_	-	-	_	0	0	4
651	17:41	4	2121190	10	212115	10	212113	300	_	_	_	_	0	0	4
651 (SB)	18:11	4	212190	10	212115	10	212113	300	0	0	0	0	0	0	4
733	16:14	1	212114	300	-	-	-	-	-	-	-	-	0	0	4
	- 6.7														

K. 2021 AM & PM STFM Link Flow Plots & Cordon Matrices

Table K1: STFM 2021 AM Peak 2-hour Cordon Matrices (Vehicles)

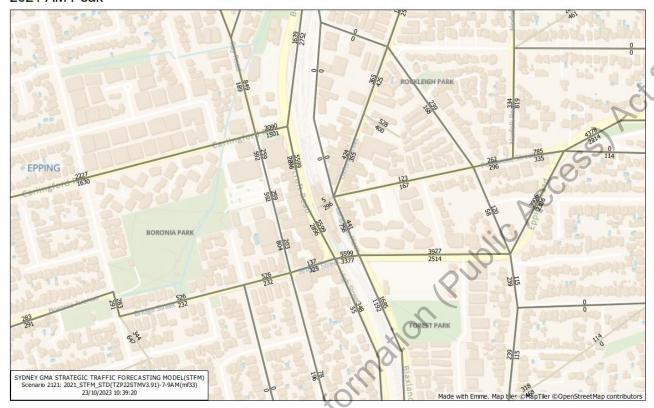
ID	1	2	3	4	5	6	7	8	9	10	11	12	TOTAL
1		743	59	46	1684		6		119	89	5		2752
2	698		37	72				1	318	36		30	1192
3	106	94			144		1		24	142		15	526
4	30	152			1793		7		207			38	2227
5	585	11	86	1096			93	337		57		41	2306
6	51		6	51				3		5		4	120
7	70		8	82		58		7		9	110	5	239
8	55										9		55
9		437					4			0			441
10	31	21	16	40	27		1		7		31	13	187
11		2			46		1		32	73		44	196
12	14	224	19	243	211		2	,	51	44	42		849
TOTAL	1639	1685	232	1630	3906	58	115	348	756	455	78	189	11091

Table K2: STFM 2021 PM Peak 2-hour Cordon Matrices (Vehicles)

ID	1	2	3	4	5	6	7	8	9	10	11	12	TOTAL
1		619	43	74	680	44	173		94	36	31		1794
2	896		165	140				3	424	24		92	1744
3	16	16		.0	115	4	17		8	70		21	266
4	102	65		77	1040	38	132		100			251	1728
5	1511		92	1479				199		23		52	3356
6	87		37	116				23		5		24	293
7	43	3	8	24	70	41		4		4		4	198
8	120	4											120
9	7	512					107						618
10	114	39	59	94	81	6	28		12		80	42	554
11		2			27	1	4		5	41		51	131
12	21	85	23	149	161	10	36		17	23	63		588
TOTAL	2909	1338	426	2077	2174	145	495	228	661	226	174	537	11390

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2021 AM Peak



2021 PM Peak



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Options Testing Technical Note

Epping Bridge Upgrade

Project: Epping Bridge Replacement

Our reference: 703100907 Your reference: EBP150523-MOTTM-

EPG-TF-RPT-000004

Rev E OFFICIAL

Prepared by: Date: 9 April 2025

Approved by: Checked by:

Subject: Options Testing Technical Note

1 Introduction

1.1 Background

This technical note documents the end-state and construction staging models developed using VISSIM and SIDRA for the Concept Design phase of the Epping Bridge project. This note serves as a supplement to our previously submitted Base Year Technical Note on February 29, 2024.

Traffic modelling is required to demonstrate the operational impacts of the project on the transport network, during construction and at end state, for the Review of Environmental Factors (REF).

This Technical Note has been structured in line with TTD2017/001.

1.2 Project Objectives

The proposed project will replace and widen the existing Epping Bridge across the combined T9 and Metro rail corridor. It will include the following upgrades to Epping Road and Beecroft Road:

- an additional eastbound right turn lane from Epping Bridge into Blaxland Road.
- an additional westbound lane across the bridge deck.
- widening of the traffic lanes on the bridge deck, continuing around the sharp curve in the Beecroft Road alignment, immediately west of the bridge.
- adjustments to the Beecroft Road access with Bridge Street and High Street.
- improved active transport facilities along the southern footpath of the bridge.
- relocation of existing Beecroft Road bus stops north of the pedestrian overbridge.

This document is issued for the party which commissioned it and for specific purposes connected with the above-captioned project only. It should not be relied upon by any other party or used for any other purpose.

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1.3 Scope of Work

Mott MacDonald has been engaged to develop a Preliminary Design Review (Concept Design) and develop the Review of Environmental Factors (REF) which will be used to seek Planning Approval for the project. The Concept Design would then be used as the reference design for the next phase of procurement for the Detailed Design and construction of the works.

The concept design will be built upon the work completed in 2021 by Future Rail for the Definition Design of the Epping Bridge Project. The Future Rail Definition Design assessed both a widening and replacement option which was used to prepare the Final Business Case, which is now complete.

The purpose of the VISSIM and SIDRA modelling is to:

- Provide traffic analysis for the Review of Environmental Factors (REF) of the Epping Bridge Replacement proposal. The modelling will need to compare traffic performance With and Without Project at opening year 2029 and With Project 10-years after opening.
- Assess the travel delay impacts of key traffic staging arrangements during construction of the project, to inform the Road User Delay Management Strategy.

Whilst there may be minor design optimisation undertaken during the Concept Design phase, the design layouts were specified in the Concept Design brief. Optioneering and/or comparative assessments of alternate designs was not included in the Concept Design scope.

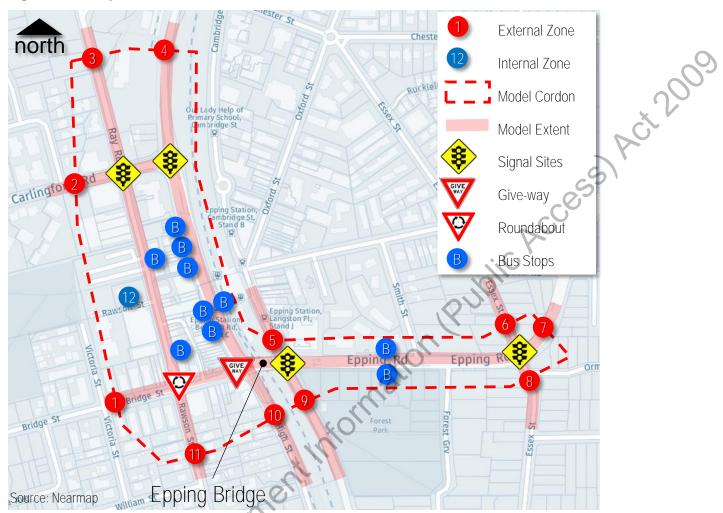
1.4 **Study Area**

The proposed extent of the VISSIM and SIDRA models is presented in Figure 1-1. The 12x12 cordon zones, .I) Government that were adopted for the previous VISSIM modelling during the Reference Design phase, are retained. The zone descriptions include:

- 7. Epping Road (east)
- 8. Essex Street (north)
- Rawson Street (south)
- 10.Blaxland Road (south)
- 11. High Street (south)
- 12. Rawson Street Carpark

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Figure 1-1 Study Area



1.5 Report Outline

This report has been structured in accordance with TTD 2017/001 and includes:

Section 1 - Introduction

Section 2 - Construction Scenario Testing

Section 3 – Future Options Testing

Section 4 - Summary and Conclusion

1.6 This revision

This revision of the technical note updates the SIDRA modelling following calibration refinements to the Base Case SIDRA models. The SIDRA analysis in Sections 2.3.4, 3.3.5 and 3.3.6 have all been updated.

Additionally, a series of alternate signal optimisation strategies have been applied to the 'With Project' scenarios, in both the VISSIM and SIDRA models, to maximise project benefits.

Both updates were undertaken in response to TfNSW comments raised on 11 February 2025.

Construction Scenario Testing 2

The objective of construction scenario modelling is to assess the potential impacts of construction works on Access Act 200's road users and identify opportunities to minimise road user delays during construction (PS251.07). The performance of the road network during the construction traffic stages was assessed using the VISSIM and SIDRA modelling software.

2.1 **Scenario Descriptions**

The following construction traffic stages were modelled:

- Stage 1A
- Stage 4A
- Stage 5B
- Stage 5B (80% Demand)
- Stage 6

These stages were selected for analysis, as they represented a significant change in layout or capacity, compared to a previous stage.

Stage 5B was modelled with normal demand and haulage then again with 80% demand, to reflect the lower demand experienced over the Christmas and New Year shut-down periods.

A review of permanent count data (Site: 74453) on the Epping Bridge over Christmas 2017, revealed that daily traffic demand dropped to 80% of normal flow over the 4-week period commencing 18 December 2017.

2.1.1 Stage 1A

It is important to maintain three lanes on the northbound Blaxland Road approach to TCS216 during construction of the retaining wall and bridge abutments on the south-west corner of the site. This will be achieved by shifting the three lanes east and reducing the southbound carriageway of Blaxland Road to a single lane.

On the northern side of the intersection, the Langston Place approach will need to be reconfigured to a dedicated left lane and a single through lane. The alignment of the southbound movement from Langston Place to Blaxland Road may be problematic due to the reverse curves through the intersection. Capacity for through movements on the Langston Place approach will be reduced during this stage.

It is understood that all signalised pedestrian crossings will be maintained during this stage.

The construction staging 1A drawing is presented in Appendix A-1.

2.1.2

The Blaxland Road approach to TCS216 is shifted west into its ultimate configuration. The Langston Place approach is also changed to its ultimate configuration and the two departure lanes on Blaxland Road are reopened.

The westbound Epping Road approach is realigned and A-detectors 2 and 3 relocated.

It is understood that all signalised pedestrian crossings will be maintained during this stage.

The construction staging 4A drawing is presented in **Appendix A-2**.

2.1.3 Stage 5B

Stage 5B reduces Epping Road eastbound capacity significantly over the Christmas Break.

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The eastbound shared through and left will be impacted by the closure of the left turn slip lane into Langston Place. Instead, this movement will be delayed by the left turn pedestrian protection.

The eastbound shared through and right lane will be blocked by right turn traffic in A-phase.

The only way to make this work efficiently and maximise the available capacity would be to operate Epping Road as split approaches phases.

All the eastbound left, through and right movements operate at the same time in a new A-phase.

All the westbound through and left movements then operate in an existing A-phase, at the same time. The Langston Place crossing is added to this B-phase. Given that there is no right turn conflict – it has little or no impact on the phase.

C-phase for Langston Place movements would operate as normal.

These signal modifications would help to minimise the impact and maximise the available lane capacities.

It is understood that all signalised crossings will be maintained during this stage.

The construction Stage 5B layout drawing is presented in Appendix A-3.

2.1.4 Stage 5B (80% Demand)

Same as 5B but modelled with 80% demand to reflect the lower demand experienced over the Christmas and New Year shut-down periods.

2.1.5 Stage 6

The Blaxland Road approach is in its end-state configuration. The Langston Place approach is adjusted to its end-state configuration.

The Epping Road eastern approach maintains two westbound lanes on the new alignment. The median lane provides a buffer to median construction works.

The Epping Road western approach maintains two through lanes and right turn lane on the new bridge approach vertical alignment.

The construction staging 6A drawing is presented in Appendix A-4.

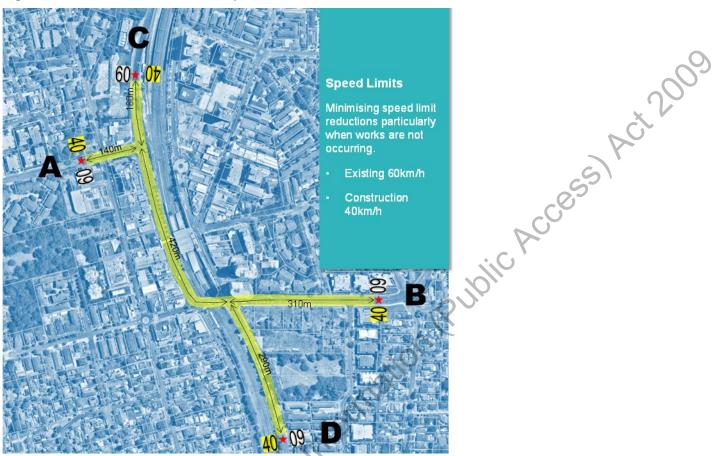
2.2 Assumptions

2.2.1 Speed Limits

The speed limits are reduced to 40km/h from the existing 60km/h in the project area during construction. The indicative extent of 40km/h construction speed limits are presented in Figure 2-1.

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Figure 2-1 Extent of Construction Speed Zones

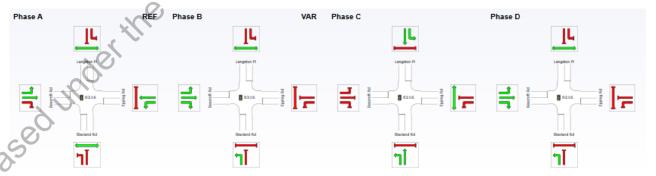


2.2.2 Signal Phasing

All existing signal phases are retained except in scenario 5B. In this 5B scenario the A, B and C phasing was adopted as shown in the Figure 2-2.

A staged pedestrian crossing is proposed on the eastern approach to the signalised intersection of Epping Road and Blaxland Road and Langston intersection place in construction stages 4A and 6.

Figure 2-2 Signal Phasing at Epping Rd / Blaxland Rd / Langston PI



2.2.3 Traffic Demand

The traffic demand data for the year 2023 was utilized for all construction scenarios , except for Stage 5B. For Stage 5B, the scenario was also modelled using 80% of the demand to account for the reduced traffic typically experienced during the Christmas and New Year shut-down periods.

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2.3 Construction Scenario Results

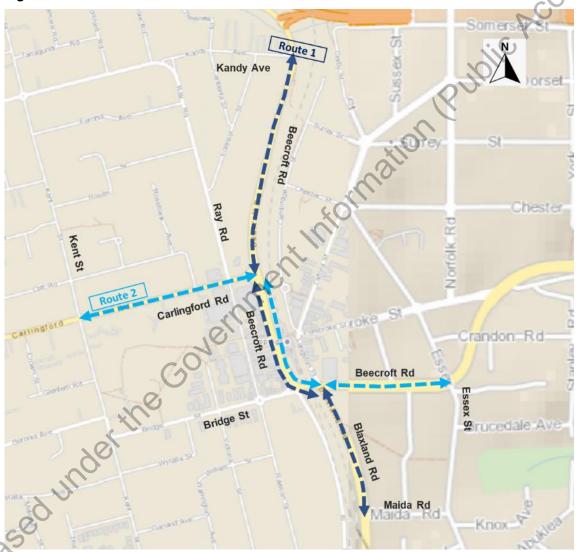
This section details the model performance in terms of travel time, network statistics, intersection performances including vehicle delay and Level of Service for all construction stages including 2023 base case.

2.3.1 VISSIM Travel Time Comparison

Travel times have been extracted for all the construction stages during AM and PM peak periods for two routes within the model area which are shown in Figure 2-3 and listed below:

- Route 1: Beecroft Road, Blaxland Road (Northbound and Southbound)
- Route 2: Carlingford Road, Beecroft Road and Epping Road (Eastbound and Westbound)

Figure 2-3 Travel Time Routes



The travel time comparison for the above-mentioned routes are provided in **Table 2-1**. Also, the cumulative travel time graphs for each route are provided in **Figure 2-4** to **Figure 2-11**.

Table 2-1 Construction Stages Travel Time Summary - AM and PM Peaks

Route	From	То	2023 Base Year	Stage 1A	Stage 4A	Stage 5B	Stage5B(80 %Demand)	Stage 6
				k (07:30-08:	30)			
	Maida Rd	Epping Rd	00:45	00:50	00:49	00:44	00:44	00:48
Beecroft Rd	Epping Rd	Carlingford Rd	00:55	01:04	01:07	01:16	01:14	01:06
(Northbound)	Carlingford Rd	Kandy Ave	00:40	00:46	00:46	00:46	00:46	00:46
	Overall		02:20	02:40	02:41	02:46	02:44	02:40
	Kandy Ave	Carlingford Rd	01:04	01:08	01:08	01:12	01:07	01:08
Beecroft Rd	Carlingford Rd	Blaxland Rd	02:34	02:36	02:31	02:17	01:11	02:10
(Southbound)	Blaxland Rd	Maida Rd	00:25	00:35	00:35	00:34	00:34	00:35
	Overall		04:03	04:19	04:14	04:03	02:51	03:53
	Kent St	Beecroft Rd	03:34	03:26	04:27	08:08	01:16	02:25
Carlingford Rd/Epping	Beecroft Rd	Blaxland Rd	00:54	00:57	00:57	01:32	01:15	00:56
Rd (Eastbound)	Blaxland Rd	Essex St	00:48	00:51	00:49	00:43	00:38	00:50
Overall	Overall		05:16	05:15	06:13	10:23	03:09	04:11
	Essex St	Blaxland Rd	01:32	01:47	01:46	02:52	01:20	01:33
Rd/Epping Rd (Westbound)	Blaxland Rd	Carlingford Rd	00:49	00:50	00:55	01:02	01:00	00:55
	Carlingford Rd	Kent St	00:46	00:51	00:51	00:43	00:41	00:52
	Overall		03:07	03:29	03:32	04:38	03:00	03:20
			PM Pea	k (17:00-18:	00)			
	Maida Rd	Epping Rd	01:11	01:18	01:26	01:12	01:04	01:14
Beecroft Rd	Epping Rd	Carlingford Rd	00:57	01:00	01:01	01:03	01:00	01:00
(Northbound)	Carlingford Rd	Kandy Ave	00:40	00:46	00:46	00:46	00:46	00:46
	Overall	No	02:48	03:04	03:13	03:01	02:50	03:00
	Kandy Ave	Carlingford Rd	00:54	00:57	00:57	00:58	00:58	00:57
Beecroft Rd	Carlingford Rd	Blaxland Rd	07:00	07:22	08:42	01:58	01:26	07:16
(Southbound)	Blaxland Rd	Maida Rd	00:25	00:35	00:35	00:34	00:34	00:35
	Overall		08:19	08:54	10:14	03:30	02:57	08:48
•	Kent St	Beecroft Rd	01:30	01:32	01:39	01:38	01:22	01:42
Carlingford Rd/Epping	Beecroft Rd	Blaxland Rd	00:40	00:44	00:44	01:22	01:07	00:44
Rd (Eastbound)	Blaxland Rd	Essex St	00:42	00:44	00:44	00:48	00:43	00:44
	Overall		02:52	03:00	03:06	03:48	03:12	03:10
7	Essex St	Blaxland Rd	01:20	01:40	01:35	01:27	01:16	01:22
Carlingford Rd/Epping	Blaxland Rd	Carlingford Rd	01:07	01:09	01:09	01:09	01:03	01:09
Rd (Westbound)	Carlingford Rd	Kent St	00:46	00:50	00:50	00:51	00:44	00:49
(55.554114)	Overall		03:13	03:39	03:35	03:27	03:03	03:20

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Figure 2-4 Construction Stage Travel Time Plot - Beecroft Rd (Northbound) - AM Peak (07:30 - 08:30)

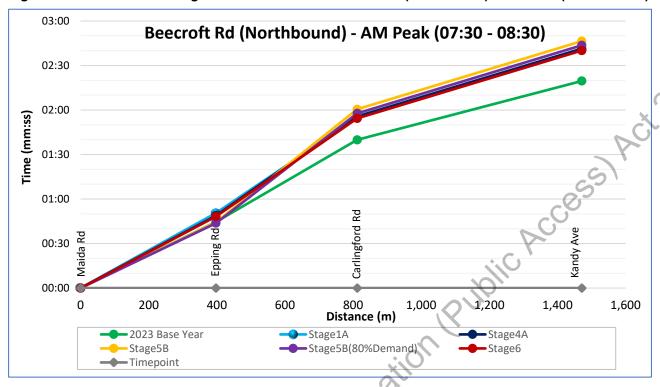
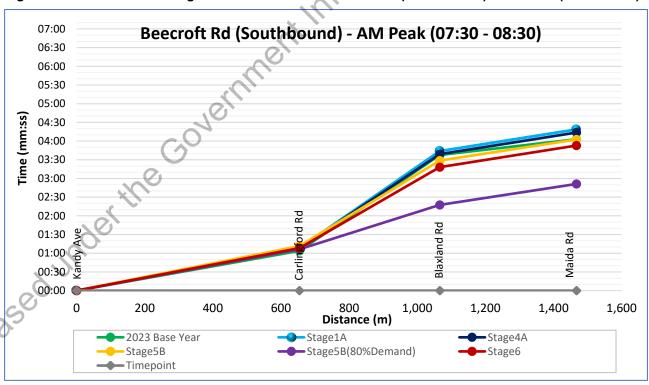


Figure 2-5 Construction Stage Travel Time Plot - Beecroft Rd (Southbound) - AM Peak (07:30 - 8:30)



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Figure 2-6 Construction Stage Travel Time Plot - Beecroft Rd (Northbound) - PM Peak (17:00 - 18:00)

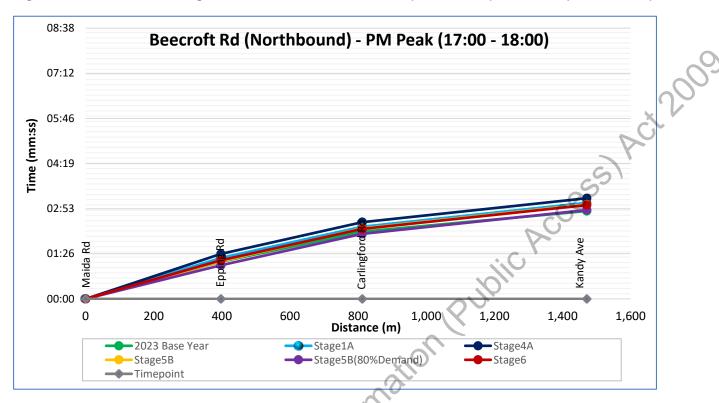
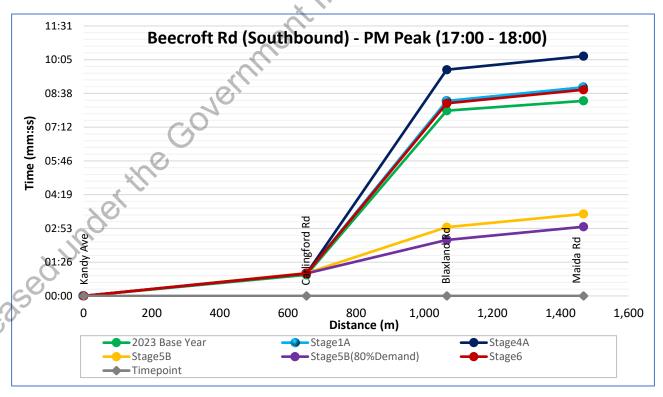


Figure 2-7 Construction Stage Travel Time Plot - Beecroft Rd (Southbound) - PM Peak (17:00 - 18:00)



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Figure 2-8 Construction Stage Travel Time Plot - Carlingford Rd/Epping Rd (Eastbound)- AM Peak (07:30 - 08:30)

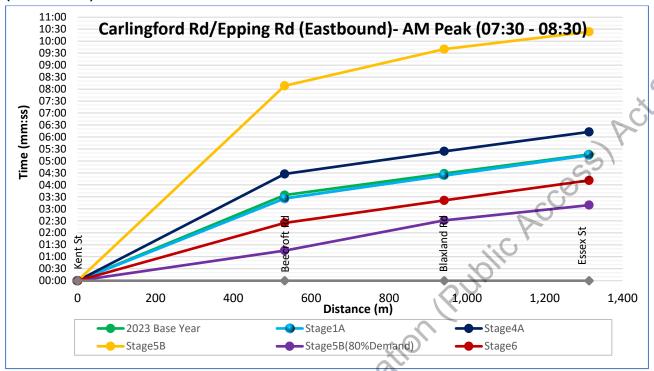
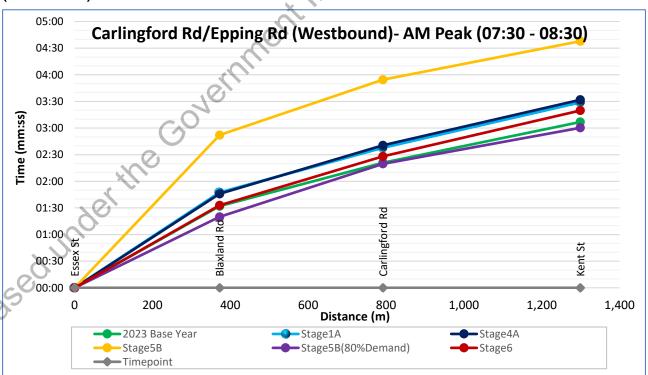


Figure 2-9 Construction Stage Travel Time Plot - Carlingford Rd/Epping Rd (Westbound)- AM Peak (07:30 - 08:30)



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Figure 2-10 Construction Stage Travel Time Plot - Carlingford Rd/Epping Rd (Eastbound)- PM Peak (17:00 - 18:00)

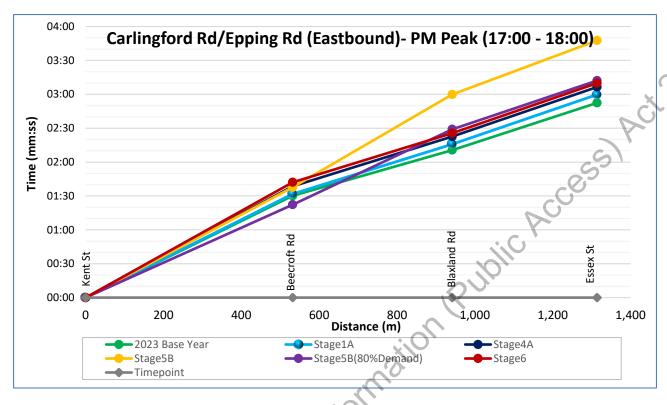
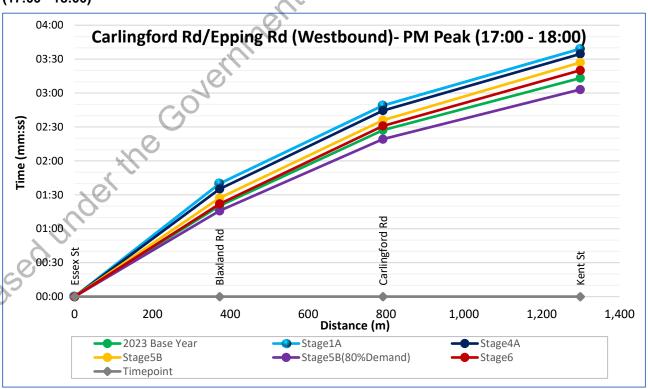


Figure 2-11 Construction Stage Travel Time Plot - Carlingford Rd/Epping Rd (Westbound)- PM Peak (17:00 - 18:00)



The key observations from the AM Peak hour VISSIM construction staging analysis include:

- The travel times along Beecroft Road northbound and southbound are approximately 10 to 30 seconds higher in all the construction stages compared to the 2023 base year except Stage 5B.
- The travel times along Carlingford Rd/Epping Rd eastbound are higher (approximately 60 seconds to 120 seconds in all the construction stages compared to the 2023 base year except Stage 1A and Stage 5B. In Stage 5B it yields 300 seconds due to a reduction in lane capacity (no dedicated right tun lane onto Blaxland Road) along Epping Bridge.
- The travel times along Carlingford Rd/Epping Rd westbound are slightly higher(approximately 10 seconds to 25 seconds) higher in all the construction stages compared to the 2023 base year except Stage 5B, which yields approximately 100 seconds.
- As expected, the travel times experienced in Stage 5B (80% Demand) are generally similar or lower than the 2023 base year.

The key observations from the PM Peak hour VISSIM construction staging analysis include:

- The travel times along Beecroft Road northbound are slightly higher (approximately 10 seconds to 25 seconds) higher in all the construction stages compared to the 2023 base year except Stage 5B.
- The travel times along Beecroft Road southbound are approximately 120 seconds higher in Stage 4A and approximately 30 seconds higher in Stage 6. All other scenarios show reduced travel times when compared to the 2023 base year.
- The travel times along Carlingford Rd/Epping Rd eastbound are slightly higher (approximately 10 seconds to 25 seconds) higher in all the construction stages compared to the 2023 base year except Stage 5B, which yields approximately 60 seconds.
- The travel times along Carlingford Rd/Epping Rd westbound are approximately 10 to 25 seconds higher in all the construction stages compared to the 2023 base year.
- As expected, the travel times experienced in Stage 5B (80% Demand) are generally similar or lower than the 2023 base year.

 As expected, the travel times experienced in Stage 5B (80% Demand) are generally similar or lower than the 2023 base year.

2.3.2 **VISSIM Network Performance Summary**

The network performance statistics indicate the efficiency of road network. For instance, models showing a cess Act 2009 comparatively higher level of vehicle stops typically represent high levels of congestion, which generally reduce throughput and increase greenhouse emissions (due to the increased stop/start nature of the driving). Conversely, a comparatively low VHT typically represents a network with a relatively low level of congestion and is generally be correlated with a low level of vehicle stops.

Table 2-2 presents the key network performance parameters for the following:

- Total distance travelled by all vehicles within the modelled network (VKT);
- Total travel time by all vehicles within the modelled network (VHT); and
- The average speed of vehicle trips on all roads within the microsimulation model;
- The average delay of vehicle trips on all roads within the microsimulation model;
- The total completed vehicle trips on all roads within the microsimulation model;
- Unreleased trips (Latent Demand) demand that could not be released in the peak hour;
- The average number of vehicle stops on all roads within the microsimulation models.

The key observations from VISSIM assessment of network performance include:

- The average network speeds are comparatively lower in both AM and PM peaks for all construction scenarios compared to the 2023 base year except Stage 5B (80% Demand). This is due to reduced speed limits in construction scenarios and network alignments mentioned in the scenario description.
- All the construction stages yield similar or slightly higher average delays when compared to the 2023 base year except Stage 5B (80% Demand).
- In the AM peak, during the construction Stage 5B there are unreleased trips due to the removal of ad a government!

 Released linder the dedicated right turn lane on to Blaxland Road at Epping Rd/ Blaxland Rd/Langston Place intersection.

Table 2-2 Construction Stages - Network Performance Summary - AM and PM Peaks

7755 263	Stage6 9645
	9645
	9645
263	
	361
29	27
71.3	89.4
4901	6122
0	4
2.14	2.67
7706	9570
254	370
30	26
65.2	94.0
5041	6266
0	0
1.83	2.50
	71.3 4901 0 2.14 7706 254 30 65.2 5041

2.3.3 **VISSIM Intersection Performance**

Table 2-3 summarises the overall intersection performance in terms of delay and LOS based on the TfNSW Access Act 2009 method, indicated below. Appendix B shows detailed results of each movement and approach wise performances.

LOS	Delay	/ (sec)				
	Low	High				
Α	0	14				
В	15	28				
С	29	42				
D	43	56				
Е	57	70				
F	>71					

The key observations from the AM Peak Hour VISSIM intersection performance assessment during construction staging, include:

- Carlingford Road / Ray Road / Rawson Street intersection yields LOS F across all scenarios except Stage 5B (80% Demand) and Stage 6, in Stage 6 it operates with LOS E.
- Bridge Street / Rawson Street and Epping Rd / Blaxland Rd / Langston Pl intersections performs at LOS E, with all other scenarios yielding LOS D or better.
- Other than above mentioned all the intersections perform satisfactorily at LOS D or better.

The key observations from the PM Peak Hour VISSIM intersection performance assessment during construction staging, include:

- Bridge Street / Rawson Street intersection performs at LOS F in Stage 5B and LOS E in Stage 6, with all other scenarios yielding LOS D or better.
- Beecroft Road / High Street / Bridge Street intersection perform at LOS F in both Stage 5B and Stage 6, with all other scenarios yielding LOS D or better.
- In Stage 4A, Epping Rd / Blaxland Rd / Langston PI intersection performs at LOS E, with all other scenarios yielding LOS D or better.
- .nen. Other than above mentioned all the intersections perform satisfactorily at LOS D or better.

Table 2-3: Intersection Performance Comparison - Construction Stages - AM and PM Peak (VISSIM)

	2023 E	Base Y	ear	St	age1A	•	St	age4A	35	Sta	age5B		Stage	5B(80% nd)	Dema	S	tage6	
Intersection Name	Total vehicl es	Avg. Dela y (S)	Lo S	Total vehicl es	Avg. Dela y (S)	Lo S	Total vehicl es	Avg. Dela y (S)	Lo S	Total vehicl es	Avg. Dela y (S)	Lo S	Total vehicl es	Avg. Delay (S)	LoS	Total vehicl es	Avg. Dela y (S)	Lo S
					A	M Pea	ak (07:30 -	08:30)										
Carlingford Rd & Ray Rd & Rawson St	2,726	79.8	F	2,756	75.2	F	2,702	88.5	F	2,417	112. 9	F	2,166	29.7	С	2,739	65.7	Е
Beecroft Rd & Carlingford Rd	4,530	24.5	В	4,564	22.1	В	4,498	22.8	В	4,213	32.7	С	3,610	25.3	В	4,543	22.8	В
Bridge Street / Rawson Street	987	40.7	С	985	39.5	С	982	54.2	D	975	66.0	E	804	29.7	С	986	46.8	D
Beecroft Road / High Street / Bridge Street	1,947	12.0	Α	1,947	16.8	В	1,929	15.7	В	1,904	19.4	В	1,590	14.4	Α	1,948	17.4	В
Epping Rd & Blaxland Rd & Langston Pl	4,817	42.8	C	4,844	41.1	С	4,781	40.1	С	4,442	60.1	E	3,849	36.3	С	4,829	34.0	С
Epping Rd & Essex St	3,237	32.3	С	3,267	28.0	В	3,223	27.1	В	2,994	27.5	В	2,599	18.8	В	3,255	27.7	В
		1			F	PM Pea	ık (17:00 -	18:00)										
Carlingford Rd & Ray Rd & Rawson St	2,726	30.6	С	2,748	31.0	С	2,729	33.8	С	2,711	34.2	С	2,180	25.2	В	2,732	33.8	С
Beecroft Rd & Carlingford Rd	4,423	28.5	В	4,458	25.7	В	4,432	24.9	В	4,414	25.8	В	3,548	23.3	В	4,433	24.5	В
Bridge Street / Rawson Street	969	45.2	D	976	46.2	D	970	39.9	С	985	91.9	F	786	33.5	С	975	61.7	E
Beecroft Road / High Street / Bridge Street	2,645	26.3	В	2,682	55.4	D	2,646	40.9	С	2,651	77.5	F	2,141	17.5	В	2,653	85.5	F
Epping Rd & Blaxland Rd & Langston Pl	4,782	51.5	D	4,822	54.8	D	4,784	57.4	Е	4,745	43.4	D	3,826	31.6	С	4,787	48.1	D
Epping Rd & Essex St	3,178	40.9	С	3,187	37.2	С	3,181	37.7	С	3,166	38.7	С	2,551	25.4	В	3,184	35.8	С

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2.3.4 SIDRA Intersection Performance

The forecast effects of the lane configuration changes during the critical construction stages was modelled in SIDRA. The SIDRA performance results for each stage, during the AM and PM peak hours, are presented in Table 2-4 and Table 2-5. The SIDRA analysis provides a sensitivity check for the VISSIM analysis.

Table 2-4 AM Peak Hour Intersection Performance During Construction

Stages	Performance Criteria	Carlingford Rd & Ray Rd & Rawson St	Beecroft Rd & Carlingford Rd	Bridge St & Rawson St	Beecroft Rd & High St	Epping Rd & Blaxland Rd	Epping Rd & Essex St
2023	DoS	1.0	0.9	0.8	0.9	1.1	0.9
(Base)	LoS	E	С	В	В	C	В
` ,	95th% Q (veh)	58	24	8	3	29	19
Stone	DoS	1.0	1.1	0.8	0.9	1.0	1.0
Stage 1A	LoS	Е	С	В	В	О В	В
	95th% Q (veh)	36	15	3	1	23	15
Ctoro	DoS	1.0	1.1	0.8	0.9	1.0	1.0
Stage 4A	LoS	E	С	В	В	В	В
	95th% Q (veh)	36	15	3	1	27	15
Ctoro	DoS	1.0	1.2	0.8	1.0	1.1	0.9
Stage 5B	LoS	E	F	В	D	Е	В
	95th% Q (veh)	36	37	3	3	33	14
Stage	DoS	0.8	0.7	0.6	0.6	1.0	0.7
5B	LoS	В	В	A	Α	В	В
(80%)	95th% Q (veh)	13	10	2	1	18	8
	DoS	1.0	1.1	0.8	0.9	1.0	1.0
Stage 6	LoS	E	C	В	В	С	С
	95th% Q (veh)	36	15	3	1	29	15

Table 2-5 PM Peak Hour Intersection Performance During Construction

Stages	Performance Criteria	Carlingford Rd & Ray Rd & Rawson St	Beecroft Rd & Carlingford Rd	Bridge St & Rawson St	Beecroft Rd & High St	Epping Rd & Blaxland Rd	Epping Rd & Essex St
2023	DoS	1.0	1.0	0.7	1.0	1.2	0.9
(Base)	LoS	С	С	Α	E	С	С
(Dase)	95th% Q (veh)	31	26	10	7	43	31
Stage	DoS	0.9	1.0	0.9	1.0	0.9	0.9
1A	LoS	С	С	Α	С	С	С
1/4	95th% Q (veh)	22	20	5	2	27	19
Stage	DoS	0.9	1.0	0.7	1.8	0.9	1.0
Stage 4A	LoS	С	С	Α	F	С	D
	95th% Q (veh)	22	18	5	10	31	19
Stone	DoS	0.9	1.1	0.7	1.1	1.0	1.4
Stage 5B	LoS	С	E	Α	F	С	F
JD	95th% Q (veh)	22	43	4	5	36	28
Stage	DoS	0.9	0.7	0.5	0.7	0.9	0.9
5B	LoS	В	В	Α	Α	В	С
(80%)	95th% Q (veh)	16	7	3	1	17	12
	DoS	0.9	1.0	0.7	1.0	0.9	1.1
Stage 6	LoS	С	D	Α	С	С	D
	95th% Q (veh)	22	40	4	2	31	20

The key observations from the SIDRA intersection performance assessment during construction staging include:

- The Carlingford Road, Ray Road and Rawson St intersection shows very little effects caused by the roadwork. The intersection LOS change from E to B in Stage 5B where 20% flow reduction is expected during the Christmas break.
- The left turn traffic from Bridge Street has a better level of service during the construction. The gap acceptance improved due to lower operation speed on Beecroft Road from 60km/hr to 40km/hr.
- In Option 5B with the lane reduction to two lanes on the west approach of the Blaxland Road intersection, the queue extends back to the Carlingford Road and Beecroft Road intersection in the morning peak. In Peleased under the Covernment information public Act the PM peak, the queue extends from Blaxland Road to Essex Road intersection with the tidal flow westbound.
 - The operation becomes acceptable to schedule Stage 5B during the Christmas break with 20% flow

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3 Future Options Testing

CACCESS ACT 2009 The objective of future scenarios is to assess the traffic performance With and Without project at opening year 2029 and 10-years after opening. The future performance was assessed using the VISSIM and SIDRA modelling software.

3.1 **Scenario Description**

The following AM and PM peak scenarios were assessed along with the base case model:

- 2029 Without Project
 - Existing road network and optimised signal times
- 2029 With Project Case
 - Network upgrades
 - An additional westbound traffic lane across bridge deck
 - An additional right turn lane southbound onto Blaxland Road
 - A wider pedestrian and bicycle shared path
 - Stagged pedestrian crossing on the eastern approach of Epping Road, Blaxland Road and Langston Place intersection
 - Signal optimisation
- 2039 Without Project
 - Existing road network and optimised signal times
- 2039 With Project
 - Network upgrades same as 2029 Project Case
 - Signal optimisation

The project case drawing is presented in Appendix C

3.2 **Assumptions**

3.2.1 **Future Traffic Demand**

The future traffic growth has been estimated based on available STFM outputs for the study area. To develop demand forecasts for 2029 and 2039, STFM's future zone to zone growth rates are applied to the existing travel patterns. This method ensures that zone-to-zone growth is considered, providing an accurate representation of the travel patterns.

Future Active Transport Demand 3.2.2

- A factor of 1.08 for 2029 and 1.18 for 2039 is applied to existing pedestrian flows.
- A factor of 1.07 for 2029 and 1.19 for 2039 is applied to existing cycles.

Signal Phasing

Signal optimization was tested for all future scenarios in the SIDRA models. The updated signal offsets and green splits were initially adopted in the VISSIM models as a starting point, and then refined. A key strategy to maximising network benefits was to favour inbound (eastbound) coordination along Carlingford Road and Beecroft Road, even during the PM peak. Outbound coordination was less effective due to the downstream bottleneck at Carlingford Road, Ray Road and Rawson Street. Additionally, some of the intersections were further optimized to balance traffic congestion in the network wherever applicable.

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3.3 **Future Year Results**

This section details the model performance in terms of travel time, network statistics, intersection performances including delay and LOS for all future year scenarios including 2023 base case.

Route	From	То	2023 Base	2029 Without	2029 With	2039 Without	2039 With
Noute	110111	10	Year	Project	Project	Project	Project
	Maida Rd	Epping Rd	00:45	00:45	00:44	00:45	00:44
Beecroft Rd	Epping Rd	Carlingford Rd	00:55	00:55	00:54	00:57	00:55
(Northbound)	Carlingford Rd	Kandy Ave	00:40	00:40	00:40	00:40	00:40
		Total	02:20	02:19	02:17	02:21	02:19
	Kandy Ave	Carlingford Rd	01:04	01:04	01:05	01:05	01:06
Beecroft Rd	Carlingford Rd	Blaxland Rd	02:34	02:47	01:11	02:21	01:10
(Southbound)	Blaxland Rd	Maida Rd	00:25	00:25	00:25	00:25	00:25
•		Total	04:03	04:16	02:41	03:51	02:41
	Kent St	Beecroft Rd	03:34	05:02	01:27	04:25	01:27
Carlingford Rd/Epping	Beecroft Rd	Blaxland Rd	00:54	00:55	00:56	00:56	00:58
Rd/Epping Rd (Eastbound)	Blaxland Rd	Essex St	00:48	00:49	00:42	00:47	01:13
(Laotbouria)		Total	05:16	06:46	03:05	06:08	03:39
	Essex St	Blaxland Rd	01:32	01:31	01:33	01:36	01:38
Carlingford Rd/Epping	Blaxland Rd	Carlingford Rd	00:49	00:53	00:49	00:51	00:54
Rd (Westbound)	Carlingford Rd	Kent St	00:46	00:48	00:45	00:48	00:46
` ,	(Total	03:07	03:12	03:07	03:15	03:18
seduni	Ser ille						

Table 3-2 Future year Scenario travel Time Summary - PM Peaks

Route From To Base Year Without Project With Project With Project Project With Project Project Project With Project Project Project With Project Project Project Project Project Project With Project Project Project Project Project Project Project Project Project Projec	' <u>'</u>							
Maida Rd Epping Rd O1:11 O1:09 O1:04 O3:01 O1:38		_	_					2039
Beecroft Rd (Northbound) Epping Rd Carlingford Rd 00:57 01:00 00:56 01:12 01:12	Route	From	10					
Epping Rd Carlingford Rd O0:57 O1:00 O0:56 O1:12 O1:12 O1:12 O1:12 O1:13 O1:04 O0:40 O0:40 O0:39 O0:39 O0:33 O0:35 O1:05		Maida Rd	Epping Rd					
Carlingford Rd Kandy Ave O0:40 O0:40 O0:39 O0:39								
Name			-					
Randy Ave Carlingford Rd 00:54 01:05 00:56 03:26 00:58 Carlingford Rd Blaxland Rd 07:00 09:49 01:05 12:31 01:08 Carlingford Rd Blaxland Rd 00:25 00:25 00:25 00:25 00:25 Total 08:19 11:18 02:26 16:22 02:33 Carlingford Rd/Epping Rd (Eastbound) Blaxland Rd 00:40 00:43 00:44 00:47 00:45 Total 02:52 05:21 02:51 08:37 07:22 Essex St Blaxland Rd 01:20 01:23 00:43 02:39 00:44 Carlingford Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:18 Total 03:13 03:10 02:27 05:03 02:54 Total 03:13 03:10 02:27 03:04 Total 03:13 03:10 02:27 03:04 Total 03:13 03:10 02:27 03:04 Total 03:13 03:10 03:10 03:10 Total 0		- Carningiola Na	•					
Carlingford Rd Blaxland Rd 07:00 09:49 01:05 12:31 01:06 Blaxland Rd Maida Rd 00:25 00:25 00:25 00:25 00:25 Total 08:19 11:18 02:26 16:22 02:33 Carlingford Rd/Epping Rd (Eastbound) Blaxland Rd 00:40 00:43 00:44 00:47 00:45 Total 02:52 05:21 02:51 08:37 07:22 Carlingford Rd/Epping Rd (Hosping Rd (Hos		IZ						
Blaxland Rd Maida Rd 00:25 00:25 00:25 00:25 00:25 Total 08:19 11:18 02:26 16:22 02:33 Kent St Beecroft Rd 01:30 03:59 01:38 07:16 06:11 Carlingford Rd/Epping Rd (Eastbound) Blaxland Rd 00:40 00:43 00:44 00:47 00:45 Total 02:52 05:21 02:51 08:37 07:27 Essex St Blaxland Rd 01:20 01:23 00:43 02:39 00:47 Carlingford Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:18 Carlingford Rd Kent St 00:46 00:44 00:45 00:49 00:45 Total 03:13 03:10 02:27 05:03 02:54 Total 03:13 03:10 03:10 03:10 03:10 Total 03:13 03:10 03:10 03:10 03:10 Total 03:13 03:10 03:10 03:10 Total 03:13 03:10 03:10 03:10 03:10 Total 03:1			-					
Total				07:00		01:05		01:08
Kent St Beecroft Rd 01:30 03:59 01:38 07:16 06:11 Carlingford Rd/Epping Rd (Eastbound) Beecroft Rd Blaxland Rd 00:40 00:43 00:44 00:47 00:45 Total 02:52 05:21 02:51 08:37 07:22 Carlingford Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:20 01:23 00:43 02:39 00:47 Carlingford Rd (Westbound) Carlingford Rd 01:07 01:03 00:59 01:34 01:15 Total 03:13 03:10 02:27 05:03 02:54 Total 03:13 03:10 02:27 05:03 02:54 Carlingford Rd Carlingford Rd 03:13 03:10 02:27 05:03 02:54 Carlingford Rd Carlingford Rd 03:13 03:10 02:27 05:03 02:54 Carlingford Rd Carlingford Rd 03:13 03:10 02:27 05:03 02:54 Carlingford Rd Carlingford Rd 03:13 03:10 02:27 05:03 02:54 Carlingford Rd Carlingford Rd 03:13 03:10 02:27 05:03 02:54 Carlingford Rd Carlingford Rd 03:13 03:10 03:13 03:10 03:13 Carlingford Rd Carlingford Rd 03:13 03:10 03:13 03:10 03:13 Carlingford Rd Carlingford Rd 03:13 03:10 03:13 03:10 Carlingford Rd Carlingford Rd 03:13 03:10 03:13 03:10 Carlingford Rd Carlingford Rd 03:13 03:10 03:13 Carlingford Rd Carlingford Rd 03:13 03:13 Carling	(Southbound)	Blaxland Rd	Maida Rd	00:25	00:25	00:25	00:25	00:25
Carlingford Rd/Epping Rd (Eastbound) Beecroft Rd Blaxland Rd 00:40 00:43 00:44 00:47 00:43 Carlingford Rd (Eastbound) Blaxland Rd Essex St 00:42 00:38 00:29 00:34 00:28 Carlingford Rd (Eastbound) Total 02:52 05:21 02:51 08:37 07:22 Essex St Blaxland Rd 01:20 01:23 00:43 02:39 00:47 Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:19 Rd (Westbound) Carlingford Rd Kent St 00:46 00:44 00:45 00:49 00:48 Total 03:13 03:10 02:27 05:03 02:54			Total	08:19	11:18	02:26	16:22	02:33
Rd/Epping Rd (Eastbound) Beech of Rd Blaxland Rd 00:40 00:43 00:44 00:47 00:47 Carlingford Rd (Eastbound) Blaxland Rd Essex St 00:42 00:38 00:29 00:34 00:28 Carlingford Rd (Eastbound) Total 02:52 05:21 02:51 08:37 07:22 Essex St Blaxland Rd 01:20 01:23 00:43 02:39 00:47 Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:15 Rd (Westbound) Total 03:13 03:10 02:27 05:03 02:54		Kent St	Beecroft Rd	01:30	03:59	01:38	07:16	06:11
Rd (Eastbound) Blaxland Rd Essex St 00:42 00:38 00:29 00:34 00:28 Carlingford Rd/Epping Rd (Westbound) Essex St Blaxland Rd 01:20 01:23 00:43 02:39 00:47 Carlingford Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:15 Total 03:13 03:10 02:27 05:03 02:54		Beecroft Rd	Blaxland Rd	00:40	00:43	00:44	00:47	00:43
Total 02:52 05:21 02:51 08:37 07:22	Rd	Blaxland Rd	Essex St	00:42	00:38	00:29	00:34	00:28
Carlingford Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:19 Total 00:46 00:44 00:45 00:49 00:45 Total 03:13 03:10 02:27 05:03 02:54	(Easibound)		Total	02:52	05:21	02:51	08:37	07:22
Carlingford Rd/Epping Rd (Westbound) Blaxland Rd Carlingford Rd 01:07 01:03 00:59 01:34 01:19 Total (Westbound) Total (Westbound) Total (Westbound) Total (Westbound) Total (Westbound) O3:10 02:27 05:03 02:54		Essex St	Blaxland Rd	01:20	01:23	1 X 	02:39	00:47
Rd (Westbound) Carlingford Rd Kent St 00:46 00:44 00:45 00:49 00:45 Total 03:13 03:10 02:27 05:03 02:54 Total 03:13 03:10 02:27 05:03 02:54 Total 03:13 03:10 03:10 03:10 03:10 Total 03:13 03:10 03:10 03:10 Total 03:13 03:10 03:10 03:10 Total 03:13 03:10 03:10 03:10 03:10 Total 03:13 03:10					01:03		01:34	01:19
(Westbound) Total 03:13 03:10 02:27 05:03 02:54	Rd		-		• ()			
Total 03:13 03:70 02:27 05:03 02:5	(Westbound)				$-\alpha$			
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Figure 3-1 Future Scenario Travel Time Plot - Beecroft Rd (Northbound) - AM Peak (07:30 - 08:30)

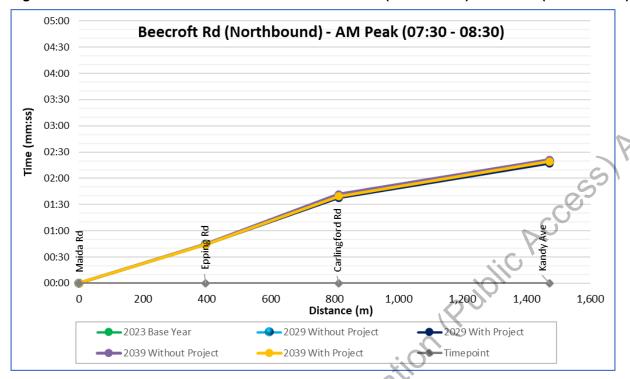
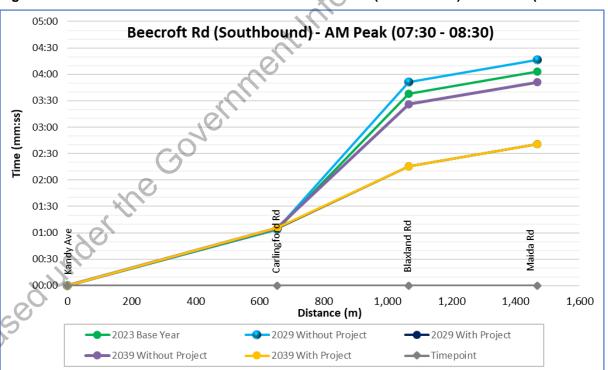


Figure 3-2 Future Scenario Travel Time Plot - Beecroft Rd (Southbound) - AM Peak (07:30 - 08:30)



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Figure 3-3 Future Scenario Travel Time Plot - Beecroft Rd (Northbound) - PM Peak (17:00 - 18:00)

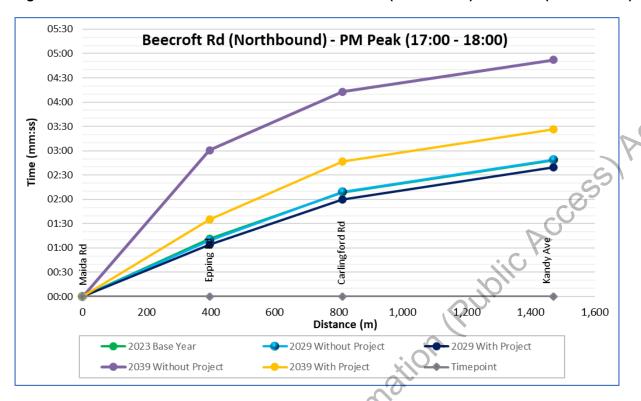
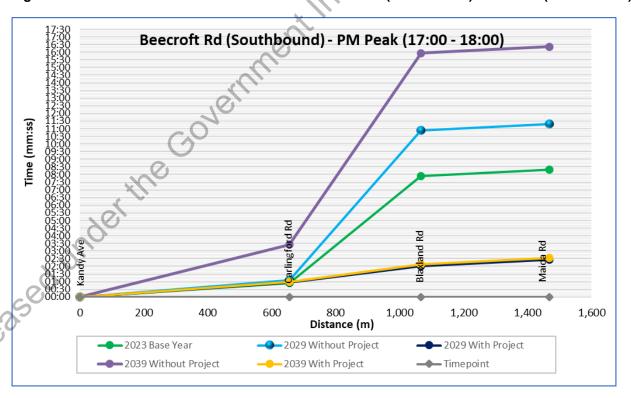


Figure 3-4 Future Scenario Travel Time Plot - Beecroft Rd (Southbound) - PM Peak (17:00 - 18:00)



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Figure 3-5 Future Scenario Travel Time Plot - Carlingford Rd/Epping Rd (Eastbound)- AM Peak (07:30 - 08:30)

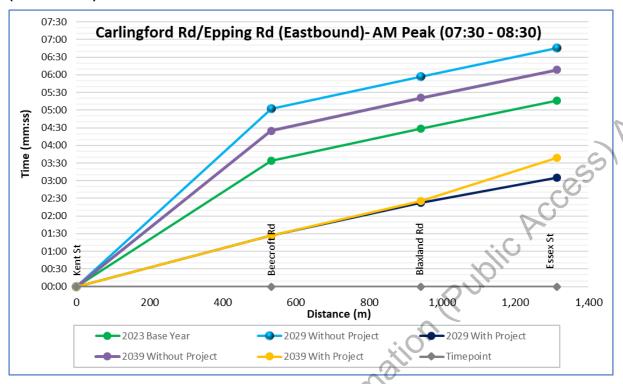
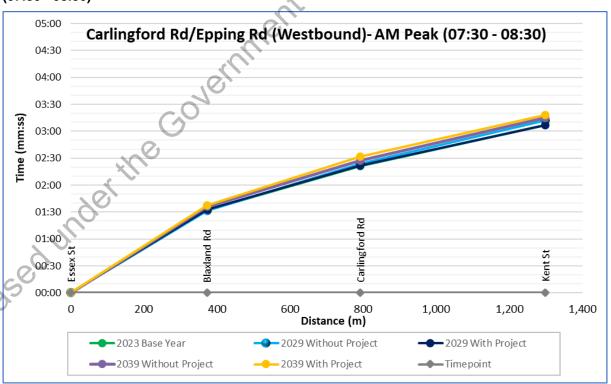


Figure 3-6 Future Scenario Travel Time Plot - Carlingford Rd/Epping Rd (Westbound)- AM Peak (07:30 - 08:30)



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Figure 3-7 Future Scenario Travel Time Plot - Carlingford Rd/Epping Rd (Eastbound)- PM Peak (17:00 - 18:00)

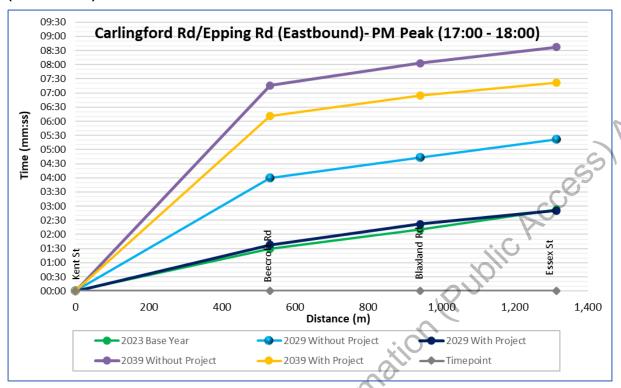
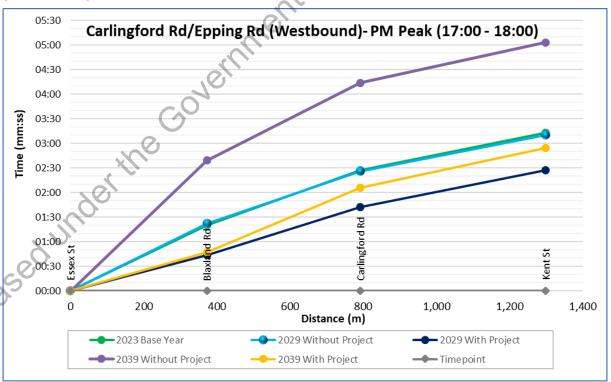


Figure 3-8 Future Scenario Travel Time Plot - Carlingford Rd/Epping Rd (Westbound)- PM Peak (17:00 - 18:00)



The key observations from AM Peak VISSIM travel time analysis include:

- Travel times along Beecroft Road northbound are similar between the 'Without Project' and 'With Project' case scenarios for the years 2029 and 2039.
- Beecroft Road southbound travel times have improved by approximately one minute and 30 seconds in 2029 and by one minute in 2039 in the 'With Project' case scenarios compared to the 'Without Project' case scenarios.
- Carlingford Road and Epping Road eastbound travel times have improved by approximately three
 minutes and 30 seconds in 2029 and by two minutes and 30 seconds in 2039 in the 'With Project' case
 scenarios compared to the 'Without Project' case scenarios.
- Travel times along Carlingford Road and Epping Road westbound are similar between the 'Without Project' and 'With Project' case scenarios for the years 2029 and 2039.

The key observations from the PM Peak VISSIM travel time analysis include:

- Travel times along Beecroft Road northbound are similar between the 'Without Project' and 'With Project' scenarios for the years 2029 and 2039.
- Beecroft Road southbound travel times have improved by approximately nine minutes in 2029 and by 13
 minutes and 30 seconds in 2039 in the 'With Project' case scenarios compared to the 'Without Project'
 case scenarios.
- Carlingford Road and Epping Road eastbound travel times have improved by approximately two minutes 30 seconds in 2029 and by one minute in 2039 in the 'With Project' case scenarios compared to the 'Without Project' case scenarios.
- Carlingford Road and Epping Road westbound travel times have improved by approximately one minute in 2029 and by two minutes in 2039 in the 'With Project' case scenarios compared to the 'Without Project' case scenarios.

 Carlingford Road and Epping Road westbound travel times have improved by approximately one minute in 2029 and by two minutes in 2039 in the 'With Project' case scenarios compared to the 'Without Project' case scenarios.

 Carlingford Road and Epping Road westbound travel times have improved by approximately one minute in 2029 and by two minutes in 2039 in the 'With Project' case scenarios compared to the 'Without Project' case scenarios.

 Carlingford Road and Epping Road westbound travel times have a scenarios compared to the 'Without Project' case scenarios compared to the 'Without Project' case scenarios.

 Carlingford Road and Epping Road westbound travel times have a scenarios compared to the 'Without Project' case scenarios cas

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3.3.2 VISSIM Network Performance

The key network performance statistics, during AM and PM peak hours, were extracted from the VISSIM models and are presented in Table 3-3 and Table 3-4 below.

Table 3-3 Forecast AM Peak Hour Network Performance

Model Performance Parameters	2023 Base Year	2029 Without Project	2029 With Project	2039 Without Project	2039 With Project
Vehicle Kilometres Travelled (km)	9651	9882	9952	10153	10437
Vehicle Hours Travelled (hours)	369	432	333	440	380
Average Speed (km/h)	26	23	30	23	27
Average Vehicle Delay (seconds)	111.9	139.5	85.9	136.1	100.7
Completed trips	6119	6290	6348	6525	6710
Unreleased trips	5	105	27	295	36
Average Number of Vehicle-stops	3.12	3.98	2.31	3.98	2.70

Table 3-4 Forecast PM Peak Hour Network Performance

Model Performance Parameters	2023 Base Year	2029 Without Project	2029 With Project	2039 Without Project	2039 With Project
Vehicle Kilometres Travelled (km)	9563	9887	9925	9687	10389
Vehicle Hours Travelled (hours)	345	422	313	875	535
Average Speed (km/h)	28	23	32	11	19
Average Vehicle Delay (seconds)	97.5	128.9	73.5	343.5	168.8
Completed trips	6252	6518	6555	6466	6965
Unreleased trips	0	0	0	515	40
Average Number of Vehicle-stops	2.29	3.10	1.85	11.75	6.37

The key observations from the VISSIM network performance analysis include:

- The average network speeds show improvement during both the AM and PM peak periods in the 'With Project' case scenarios compared to the 'Without Project' case scenarios for the years 2029 and 2039.
- The average network delay is significantly lower during both the AM and PM peak periods in the 'With Project' case scenarios compared to the 'Without Project' case scenarios for the years 2029 and 2039.
- Additionally, the number of unreleased trips is lower during both the AM and PM peak periods in the 'With Project' case scenarios compared to the 'Without Project' case scenarios for the years 2029 and 2039.
- This indicates that the proposed project will enhance traffic flow and reduce congestion during peak hours.

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3.3.3 SIDRA Network Performance

The key network performance statistics, during AM and PM peak hours, were extracted from the SIDRA models and are presented in Table 3-5 and Table 3-6. The SIDRA analysis provides a sensitivity check for the VISSIM analysis.

Table 3-5 Forecast AM Peak Hour Network Performance

Model Performance Parameters	2023 Base Year	2029 Without Project	2029 With Project	2039 Without Project	2039 With Project
Network Capacity (veh/hr)	23506	23652	23818	24622	24757
Average Vehicle Speeds (km/h)	27.8	25.1	27.3	19.3	20.8
Total travel Time (Veh-h/h)	365	404	387	550	512
Average Control Delay (sec)	26.8	32.7	28.6	51.0	45.4

Table 3-6 Forecast PM Peak Hour Network Performance

Model Performance Parameters	2023 Base Year	2029 Without Project	2029 With Project	2039 Without Project	2039 With Project
Network Capacity (veh/hr)	22994	24010	24106	25034	25155
Average Vehicle Speeds (km/h)	27.7	27.1	28.1	19.1	21.9
Total travel Time (Veh-h/h)	362	387	371	572	497
Average Control Delay (sec)	27.7	29.0	26.7	53.0	42.2

The key observations from the SIDRA network performance assessments include:

- In both peaks, the 'With Project' scenarios all provide better network performance than the 'Without Project' scenarios.
- 'Without Pproject', the network will deteriorate over time. Vehicle delay will almost double and travel speeds will slow by up to 9km/h.
- In 2029, the SIDRA analysis estimates that the project will reduce network delay by up to 13% and 8%, in the AM and PM peak, respectively.
- By 2039, the benefits of the project are even greater with the bridge widening reducing network delay by up to 11% and 20%, in the AM and PM peaks, respectively.

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3.3.5 VISSIM Forecast Peak Hour Intersection Performance

The VISSIM forecast intersection performance for the AM and PM peak hours are summarised in Table 3-7 and Table 3-8. **Appendix D** provides an additional breakdown of intersection performance by intersection approach.

Table 3-7: Forecast AM Peak Hour Intersection Performance

Year	Performance Criteria	Carlingford Rd & Ray Rd & Rawson St	Beecroft Rd & Carlingford Rd	Bridge Street / Rawson Street	Beecroft Road / High Street / Bridge Street	Epping Rd & Blaxland Rd & Langston Pl	Epping Rd & Essex St
	Total Vehicles	2726	4530	987	1947	4817	3237
2023	Ave Delay (sec)	79.8	24.5	16.7	2.2	42.8	32.3
	Level of Service	F	В	В	Α	CE	С
	Total Vehicles	2882	4607	1081	2031	4880	3270
2029 Without Project	Ave Delay (sec)	83.5	25.3	24.0	2.7	43.9	50.6
	Level of Service	F	В	В	A	D	D
	Total Vehicles	2917	4659	1085	2088	4891	3332
2029 With Project	Ave Delay (sec)	43.5	23.8	17.4	3.3	33.1	34.3
	Level of Service	D	В	В	А	С	С
	Total Vehicles	3077	4653	1196	2112	4962	3297
2039 Without Project	Ave Delay (sec)	86.6	25.7	35.6	2.8	46.0	49.5
,	Level of Service		В	С	А	D	D
	Total Vehicles	3120	4785	1231	2296	5097	3492
2039 With Project	Ave Delay (sec)	50.5	24.7	27.1	4.7	37.0	44.2
	Level of Service	D	В	В	А	С	D

The key observations from the AM Peak hour VISSIM intersection performance analysis include:

- In the 'Without Project' scenarios for the years 2029 and 2039, the intersection at Carlingford Road, Ray Road, and Rawson Street operates at LOS F. However, in the 'With Project' scenarios, this intersection improves to LOS D or better.
- All intersections in the 'With Project' case scenarios for the years 2029 and 2039 show improvements compared to the 'Without Project' case scenarios for the same years
- Additionally, all intersections perform satisfactorily at LOS D or better in both the 'Without Project' case and 'With Project' case scenarios for the years 2029 and 2039.

Table 3-8 Forecast PM Peak Hour Intersection Performance

Year	Performance Criteria	Carlingford Rd & Ray Rd & Rawson St	Beecroft Rd & Carlingford Rd	Bridge Street / Rawson Street	Beecroft Road / High Street / Bridge Street	Epping Rd & Blaxland Rd & Langston Pl	Epping Rd & Essex St
	Total Vehicles	2726	4423	969	2645	4782	3178
2023	Ave Delay (sec)	30.6	28.5	11.2	2.7	51.5	40.9
	Level of Service	С	В	А	А	D	Se
	Total Vehicles	2851	4557	1043	2737	4915	3201
Project	Ave Delay (sec)	77.4	35.6	13.6	3.3	53.9	38.5
	Level of Service	F	С	А	А	(CD	С
2029	Total Vehicles	2897	4594	1042	2717	4948	3219
With Project	Ave Delay (sec)	35.9	26.4	13.6	2.1	25.1	34.0
	Level of Service	С	В	A	А	В	С
	Total Vehicles	2642	4422	1084	2808	4878	3142
2039 Without Project	Ave Delay (sec)	113.0	70.2	263.9	13.9	89.6	54.9
	Level of Service	F	F, N	F	Α	F	D
	Total Vehicles	Vehicles 2985		1210	2901	5135	3269
Project	Ave Delay (sec)	83.6	35.0	119.7	11.4	33.3	34.8
	Level of Service	FO	С	F	А	С	С

The key observations from the PM Peak hour VISSIM intersection performance analysis include:

- The intersections at Carlingford Road, Ray Road, and Rawson Street intersection, perform at LOS F in both the 'Without Project' and 'With Project' scenarios for the years 2029 and 2039, except for the 2029 'With Project' scenario. However, the 'With Project' scenarios show improvements in terms of delays compared to the 'Without Project' scenarios.
- The Bridge Street and Rawson Street intersection perform at LOS F in both the 'Without Project' and 'With Project' scenarios in the year 2039.
- The Epping Road, Blaxland Road and Langston Place, along with Beecroft Road and Carlingford Road intersection, perform at LOS F in 'Without Project' case scenarios for the year 2039. However, the 'With Project' case scenarios show improvements in terms of delays.
- Other than the intersections mentioned above, all other intersections perform satisfactorily at LOS D or better in both the 'Without Project' and 'With Project' case scenarios for the years 2029 and 2039.

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3.3.6 SIDRA Forecast Peak Hour Intersection Performance

The SIDRA forecast intersection performance for the AM and PM peak hours are summarised in Table 3-9 and Table 3-10. The SIDRA analysis provides a sensitivity check for the VISSIM analysis.

Table 3-9: Forecast AM Peak Hour Intersection Performance

Year	Performance Criteria	Carlingford Rd & Ray Rd & Rawson St	Beecroft Rd & Carlingford Rd	Bridge St & Rawson St	Beecroft Rd & High St & Bridge St	Epping Rd & Blaxland Rd	Epping Rd & Essex St
2023	DoS	1.0	0.9	0.8	0.9	1.1	0.9
	LoS	E	С	В	В	C	ЭВ
	95th Q (veh)	58	24	8	3	29	19
2029	DoS	1.1	0.9	0.9	1.1	1.0	1.1
Without Project	LoS	F	В	В	F	В	D
1 10,000	95th Q (veh)	42	13	5	7	29	22
2029	DoS	1.1	0.9	0.9	0.6	0.9	1.0
With Project	LoS	F	С	В	А	В	D
1 10,000	95th Q (veh)	42	13	6	1	9	22
2039	DoS	1.2	1.1	1.0	1.3	1.1	1.4
Without Project	LoS	F	С	EX	F	С	F
1 10,000	95th Q (veh)	37	31	11	10	26	38
2039 With	DoS	1.2	1.1	1.0	0.6	0.9	1.4
Project	LoS	F	D C	E	Α	В	F
	95th Q (veh)	37	33	12	1	10	40

Table 3-10 Forecast PM Peak Hour Intersection Performance (SIDRA)

Year	Performanc e Criteria	Carlingford Rd & Ray Rd & Rawson St	Beecroft Rd & Carlingford Rd	Bridge St & Rawson St	Beecroft Rd & High St & Bridge St	Epping Rd & Blaxland Rd	Epping Rd & Essex St
2023	DoS	1.0	1.0	0.7	1.0	1.2	0.9
	LoS	C	С	Α	E	С	С
	95th Q (veh)	31	26	10	7	43	31
2029	DoS	0.9	1.0	1.1	1.3	0.9	0.9
Without Project	LoS	С	D	В	F	В	D
i iojout	95th Q (veh)	40	62	17	24	42	34
2029	DoS	0.9	1.0	0.9	0.4	0.9	0.9
With Project	LoS	С	D	В	Α	В	D
Signature	95th Q (veh)	40	65	12	1	28	34
2039	DoS	1.0	1.1	1.2	2.7	1.0	0.9
Without Project	LoS	D	E	E	F	С	D
1 10,000	95th Q (veh)	63	69	38	25	45	37
2039 With	DoS	1.0	1.1	1.0	0.7	0.9	0.9
Project	LoS	E	F	С	Α	В	D
	95th Q (veh)	65	76	17	2	34	37

The key observations from the SIDRA intersection performance analysis include:

- Carlingford Road, Ray Road and Rawson Street becomes LOS F in all future year AM Peak scenarios.
 The LOS becomes E in the 2039 PM peak. The intersection does not benefit from the upgrade due to the constraint of limited spaced to Beecroft Road and Carlingford Road intersection.
- Beecroft Road & Carlingford Road will reach its capacity in 2029, as show in the PM peak Table 3-10.
 The LOS deteriorates to F in 2039 PM peak in the project scenario due to higher northbound arrival flows from the upgraded intersection of Epping Road, Blaxland Road and Langston Place.
- With the bridge upgrade in 2039, the overall intersection performance at Epping Road, Blaxland Road
 and Langston Place improves from LOS C to B in both peaks, compared to the 'Without Project' scenario.
 The biggest performance improvements are experienced by the eastbound right turn from Epping Bridge
 into Blaxland Road, during the AM Peak, where movement performance improves from LOS F to B.
- Despite the overall western approach of Epping Road, at Blaxland Road, operating at LOS C 'Without Project' in the 2029 AM and PM peaks, this performance is a little misleading. The heavy eastbound right turn movement from Epping Bridge into Blaxland Road is operating at LOS F.
- The model also shows Essex Street intersection will reach its capacity in 2029. The LOS deteriorates to F in 2039 AM peak suggests an upgrade is required.
- The delay on the priority control left turn from Bridge Street to Beecroft Road becomes very high (LOS F) if no bridge upgrade takes place. The opposing traffic will be too busy to find a suitable gap to proceed. This also caused queue back to the Beecroft Road, High Street and Bridge Street roundabout in 2039, which would operate at LOS E for both peaks.
- With the project upgrades, the traffic from Bridge Street can proceed with the additional short lane on Beecroft Road merge with the main road traffic. This reduces the delay significantly to less than six seconds (LOS A). This in turn releases the gridlock on Bridge St in the PM peak and improves the roundabout operation from LOS E to C.

4 Summary and Conclusion

This technical note details and summarised the traffic assessment undertaken for construction stages and project scenarios of the proposed Epping Bridge widening.

Based on the analysis and discussions presented within this report, the following is a summary of the modelling outcomes:

4.1 Travel Times

Construction Stages:

- All the construction stages yield similar or slightly increased travel times along both routes compared to the 2023 base year in both peaks.
- Beecroft Road southbound movements experienced increase in the travel time approximately by 120 seconds in the PM peak in Stage 4A compared to the 2023 base year.
- Carlingford Rd & Epping Rd eastbound movements experienced increase in the travel time approximately by 120 seconds in the AM peak in Stage 1A and Stage 5B compared to the 2023 base year.
- The travel times experienced in Stage 5B (80% Demand) are generally similar or lower than the 2023 base year.

Future Year Scenarios:

- Travel times along Beecroft Road northbound remain consistent across both AM and PM peak periods in the 'Without Project' and 'With Project' scenarios for the years 2029 and 2039.
- Beecroft Road southbound travel times have improved by approximately one minute and 30 seconds in 2029 and by one minute in 2039 in the 'With Project' scenarios compared to the 'Without Project'

scenarios during the AM peak period. During the PM peak period, Beecroft Road southbound travel times have improved by nine minutes in 2029 and by 13 minutes and 30 seconds in 2039 in the 'With Project' case scenarios compared to the 'Without Project' scenarios.

- Travel times along Carlingford Road and Epping Road eastbound have improved by approximately three minutes and 30 seconds in 2029 and by two minutes and 30 seconds in 2039 in the 'With Project' scenarios compared to the 'Without Project' scenarios during the AM peak period. During the PM peak period, Carlingford Road and Epping Road eastbound travel times have improved by two minutes 30 seconds in 2029 and by one minute in 2039 in the 'With Project' case scenarios compared to the 'Without Project' scenarios.
- Travel times along Carlingford Road and Epping Road westbound are similar between the 'Without Project' and 'With Project' scenarios for the years 2029 and 2039 in the AM peak period. During the PM peak period, Carlingford Road and Epping Road westbound travel times have improved by approximately one minute in 2029 and by two minutes in 2039 in the 'With Project' case scenarios compared to the 'Without Project' scenarios.

4.2 Network Performance

Construction Stages:

- All construction scenarios except Stage 5B (80% Demand) yield lower average network speeds and slightly higher average network delays compared to the 2023 base year in both peaks.
- In the AM peak, during the construction Stage 5B there are unreleased trips due to the eastbound lane closure at Epping Rd / Blaxland Rd / Langston PI intersection.

Future Year Scenarios:

- The average network speeds show improvement during both the AM and PM peak periods in the 'With Project' scenarios compared to the 'Without Project' scenarios for the years 2029 and 2039.
- The average network delay is significantly lower during both the AM and PM peak periods in the 'With Project' scenarios compared to the 'Without Project' scenarios for the years 2029 and 2039.
- Additionally, the number of unreleased trips is lower during both the AM and PM peak periods in the 'With Project' scenarios compared to the 'Without Project' scenarios for the years 2029 and 2039.
- This indicates that the proposed project will enhance traffic flow and reduce congestion during peak hours.

4.3 Intersections Performance

Construction Stages:

- Carlingford Road / Ray Road / Rawson Street intersection yields LOS F across all construction stages except Stage 5B (80% Demand) in the AM peak, in Stage 6 it operates with LOS E.
- Bridge Street / Rawson Street and Epping Rd / Blaxland Rd / Langston PI intersections performs at LOS E, with all other scenarios yielding LOS D or better in the AM peak.
- Bridge Street / Rawson Street intersection performs at LOS F in Stage 5B and LOS E in Stage 6, with all
 other scenarios yielding LOS D or better in the PM peak.
- In Stage 4A, Epping Rd / Blaxland Rd / Langston Pl intersection performs at LOS E, with all other scenarios yielding LOS D or better in the PM peak.
- Other than above mentioned all the intersections perform satisfactorily at LOS D or better in both peaks.

Future Year Scenarios:

- During the AM peak periods, all other intersections perform satisfactorily at LOS D or better in both the 'Without Project' and 'With Project' scenarios for the years 2029 and 2039, except for the Carlingford Road, Ray Road, and Rawson Street intersection in the 2039 'Without Project' case scenario.
- The intersections at Carlingford Road, Ray Road, and Rawson Street perform at LOS F in both the 'Without Project' and 'With Project' case scenarios for the years 2029 and 2039 during the PM peak,

except for the 2029 'With Project' scenario. However, the 'With Project' scenarios show improvements in

- wever, the 'With Project' scenail oject' scenails.

 .arsection also performs at LOS F in both intersections are Epping Road, Blaxland Road, anglord Road intersection, perform at LOS F in the a "With Project' scenarios show improvements in terms.

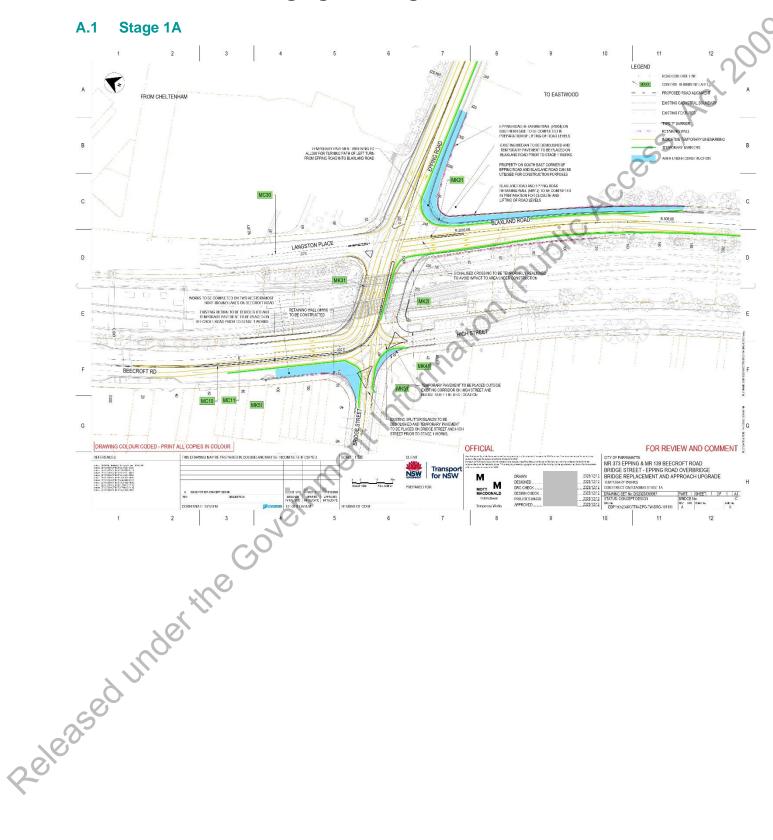
 .all intersections in the project scenarios for the years 2029 on the Without Project' scenarios for the same years.

 .been carried out during the construction stages and project scenarios etailed in the respective sections.

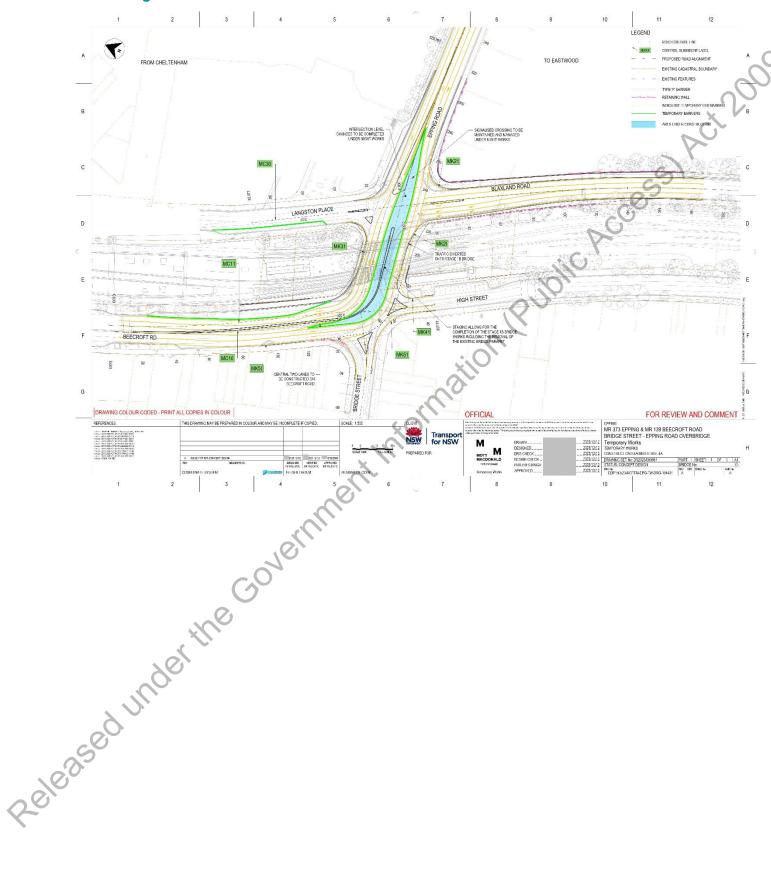
 .dening of the Epping Bridge in the project scenarios shows improved network and travel times compared to the 'Without Project' scenarios. during the PM peak. Additionally, the intersections at Epping Road, Blaxland Road, and Langston Place, along with Beecroft Road and Carlingford Road intersection, perform at LOS F in the 'Without Project'
 - Other than above mentioned, all intersections in the project scenarios for the years 2029 and 2039 show

Also, SIDRA analysis has been carried out during the construction stages and project scenarios in line with

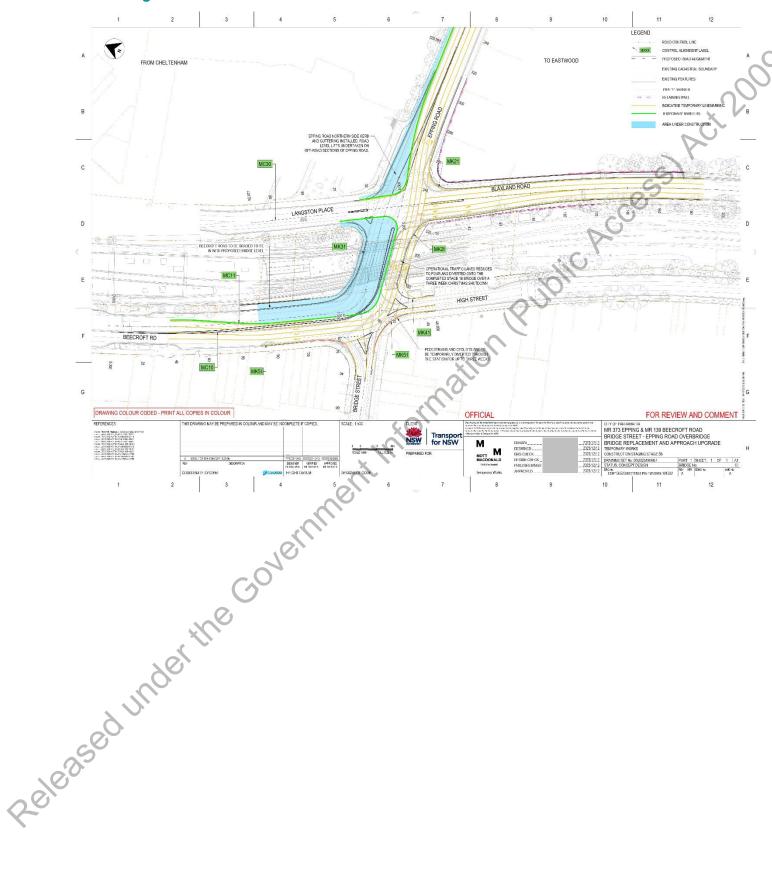
A. Construction Staging Drawings



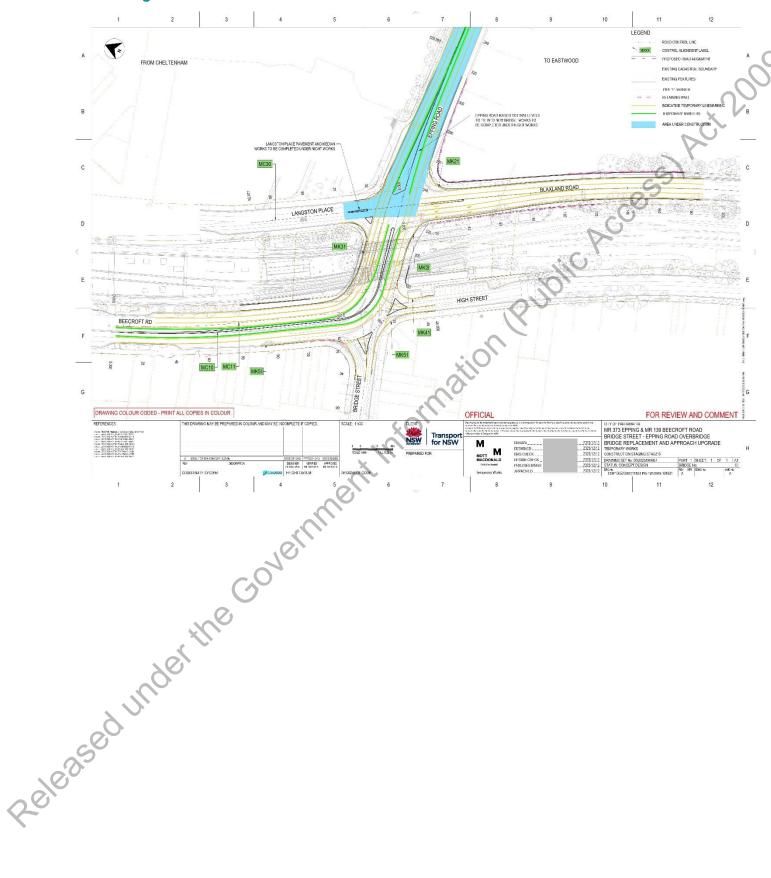
A.2 Stage 4A



A.3 Stage 5B



A.4 Stage 6



B. Intersection Performance - Construction Stages

B.1 AM Peak

Intersection Name	Approach Name	Movement Name				2023 Ba	se Year							Stag		2						Stag	;e4A			
			Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS
		Left	44	21	D	63			(,		42	18	С	63			()		45	21	D	63			()	
	South	Thru	45	21	D	141	269	73	21	F	42	18	C	141	269	63	18	Ε	47	21	D	141	269	73	21	F
		Right	162	21	F	65					131	18	AE Ø	65					156	21	F	65				
	East	Left Thru	3 10	9	A A	84 788	876	9	11	^	3 11	11	A	84 787	877	10	13	Α	4 11	12 14	A A	84 777	867	10	14	Α
Carlingford Rd & Ray Rd &	Last	Right	45	11	D	4	870	,	-11	Û	13	13	A	6	677	10	13	- î	25	13	В	6	807	10	14	n n
Rawson St		Left	78	37	F	261					73	34	J F	267					89	41	F	266				
	North	Thru	41	37	С	204	483	62	37	E	42	34	С	204	489	59	34	E	41	41	С	204	488	67	41	E
		Right	52	37	D	18					50	34	D	18					51	41	D	18				
		Left	137	234	F	7					138	224	F	7					167	313	F	7				
	West	Thru Right	146 176	234 234	F	1086 5	1098	146	234	۲	136 152	224 224	F	1109	1121	136	224	F	165 212	313 313	F	1066 5	1078	165	313	- 1
		Left	170	2	В	699					11	1	A	702					13	2	Α	691				
	South	Thru	42	42	С	969	1668	31	25	C	36	40	С	973	1675	25	24	В	36	41	С	964	1655	27	25	В
Beecroft Rd & Carlingford	North	Thru	27	26	В	1279	1452	31	24_	7	27	30	В	1278	1451	31	28	С	27	29	В	1278	1451	30	28	С
Rd	North	Right	62	14	E	173	1452	31	24	O	59	13	E	173	1451	31	28	C	58	13	E	173	1451	30	28	C
	West	Left	13	0	Α	45	1410	10	20	A	15	0	В	45	1438	10	20	Α	10	0	Α	43	1392	10	20	Α
		Right	10	20	A	1365					10	21	A	1393					10	20	A	1349				
	South	Left Thru	11 11	2	A A	12 99	191	C	2	Δ	11 9	2	A A	12 99	190	10	2	Α	13	2	A A	12 99	190	11	2	Α
	300011	Right	15	2	A	80	191	X	2	Û	11	2	A	79	150	10	2		12	2	A	79	190	-11	2	n n
		Left	7	1	A	22	11				6	1	A	22					7	1	A	22				
	East	Thru	9	1	Α	111	198	9	1	Α	8	1	Α	111	199	9	1	Α	10	1	Α	108	196	10	1	Α
Bridge Street / Rawson		Right	8	1	Α	65	X				10	1	Α	66					10	1	Α	66				
Street		Left	41	12	С	12					39	13	С	11		27 13			54	16	D	11				
	North	Thru	23 27	12	B B	117 164	293	26	12	В	25 28	13 13	В	116 162	289	27	13	В	30 35	16 16	C C	117	291	33	16	С
		Right Left	15	12 8	В	175					14	8	A	176					14	7	A	163 175				
	West	Thru	16	8	В	115	305	15	8	В	15	8	A	115	307	14	8	_	15	7	A	115	305	14	7	Α
		Right	15	8	A	15					11	8	Α	16					8	7	Α	15				
	South	Left	9	0	А	24	24	9	0	Α	13	0	Α	24	24	13	0	Α	6	0	Α	24	24	6	0	Α
Beecroft Road / High		Left	1	9	A	87					1	0	Α	86					1	1	Α	87				
Street / Bridge	East	Thru	1 4	0	Α Δ	178	1716	1	0	Α	0	0	A A	176 1454	1716	1	0	Α	1	0	A A	174 1438	1699	1	0	Α
Street	West	Right Left	12	5	A	1451 207	207	12	5	Α	17	4	B	207	207	17	4	В	16	7	B	206	206	16	7	В
		Left	18	20	В	611					18	19	В	616					18	19	В	615				
	South	Thru	57	20	E	183	794	27	20	В	50	19	D	184	800	25	19	В	50	19	D	184	799	25	19	В
	East	Left	49	75	D	12	1118	54	75	D	73	104	F	12	1114	64	106	Е	79	102	F	12	1096	62	102	Е
Epping Rd & Blaxland Rd &		Thru	54	75	D	1106					64	106	E	1102					62	102	E	1084				
Langston PI	North	Left Thru	54 48	12 12	D D	39 226	265	49	12	D	49 50	18 18	D D	39 226	265	50	18	D	45 48	11 11	D D	39 226	265	47	11	D
		Left	48 26	63	В	270					24	18 86	В	273					48 21	72	В	265				
	West	Thru	21	167	В	1820	2640	42	157	С	14	175	A	1838	2665	35	166	С	13	169	A	1805	2621	35	159	С
		Right	122	167	F	550					113	175	F	554					110	169	F	551				
	ZO	Left	53	12	D	26					53	12	D	26					53	12	D	26				
	South	Thru	63	12	E	85	185	57	12	E	63	12	E	85	185	57	12	E	63	12	E	85	185	57	12	E
		Right	52	12	D	74					52	12	D	74					52	12	D	74				
	East	Left	9	7	A	14 676	690	14	7	Α	11 14	7	A A	14 676	690	14	7	Α	11	7	A A	14 676	690	14	7	Α
Epping Rd & Essex St		Left	126	47	A	2					71	47	F	2					126	47	F	2				
	North	Thru	72	47	F	83	519	74	47	F	74	47	F	84	522	73	47	F	72	47	F	82	519	74	47	F
2.0		Right	74	47	F	434					73	47	F	436					74	47	F	435				
	West	Left	34	74	С	62	1843	25	74	В	30	65	С	62	1870	18	65	В	27	58	В	62	1829	16	58	В
		Thru	25	74	В	1781					17	65	В	1808					16	58	В	1767				

Intersection Name	Approach Name	Movement Name				Sta	ge5B							Stage5B(80	%Demand)			(i)				Sta	ge6			
			Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (S) Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS
		Left	46	22	D	63			()		40	10	С	51			()		49	27	D	63			(,	
	South	Thru	44	22	D	142	272	77	22	F	38	10	С	113	221	44	10	D	45	27	D	141	270	84	27	F
		Right	177	22	F	67					58	10	Е	57			ر (C		201	27	F	66				
		Left	2	4	А	85					2	2	А	69		-60			2	12	А	84				
	East	Thru	6	5	Α	768	859	6	5	Α	4	3	Α	621	696	73	3	Α	11	14	Α	785	875	10	14	Α
Carlingford Rd & Ray Rd &		Right	22	5	В	6					14	3	Α	6					13	13	Α	6				
Rawson St		Left	166	77	F	254					59	22	Е	214					94	44	F	260				
	North	Thru	42	77	С	204	476	109	77	F	38	22	С	166	397	50	22	D	41	44	С	204	482	70	44	F
		Right	52	77	D	18					47	22	D	17					51	44	D	18				
		Left	204	487	F	4					27	27	В	6					119	134	F	7				
	West	Thru	241	487	F	800	810	241	487	F	38	27	С	841	852	38	27	С	103	134	F	1100	1112	103	134	F
		Right	323	487	F	6					36	27	C	5					131	134	F	5				
		Left	16	11	В	682					14	6	А	554					14	1	Α	700				
	South	Thru	48	49	D	963	1645	34	33	С	47	38	D	788	1342	33	25	С	36	40	С	964	1664	27	24	В
Beecroft Rd & Carlingford		Thru	32	45	С	1275					25	27	В	1022					27	29	В	1278				
Rd	North	Right	62	12	Е	173	1448	36	41	С	59	12	ľΕ	137	1159	29	25	С	59	14	Е	173	1451	30	28	С
		Left	16	0	В	46					11 (0	Α	42					13	1	Α	45				
	West	Right	27	29	В	1074	1120	26	28	В	12	18	Α	1067	1109	12	18	Α	10	21	Α	1383	1428	10	20	Α
		Left	12	2	Α	12					_3	2	Α	10					10	2	Α	12				
	South	Thru	10	2	Α	99	190	12	2	Α	12	2	Α	79	153	12	2	Α	11	2	Α	99	190	12	2	Α
		Right	14	2	Α	79				. (14	2	Α	64					14	2	Α	79				
		Left	9	1	Α	22				1	6	1	Α	19					4	1	Α	22				
	East	Thru	9	1	Α	101	184	10	1	Α	9	1	Α	90	169	9	1	Α	7	1	А	110	198	7	1	Α
Bridge Street		Right	12	1	A	61				$\mathcal{N}_{\mathcal{P}}$	9	1	A	60					9	1	A	66				
/ Rawson Street		Left	66	26	F	12			0	0	30	5	C	11					47	11	D	11				
Street	North	Thru	45	26	D	117	294	49	26	D	14	5	A		242	15	5	В	23	11	В	117	291	24	11	В
		Right	51	26	D	165			1		16	5	В	135					23	11	В	163				
		Left	13	7	A	176		. 0			6	2	A	138					15	9	В	176				
	West	Thru	15	7	A	115	307	13	7	Α	8	2	A	91	240	7	2	Α	18	9	В	115	307	16	9	В
		Right	9	7	Α	16					6	2	A	11					13	9	A	16				_
	South	Left	15	0	A	23	23	15	0	Α	6	0	A	20	20	6	0	Α	13	0	A	24	24	13	0	A
Beecroft		Left	0	0	A	84		-	-		0	0	A	72		-	-		0	0	A	87				
Road / High	East	Thru	0	0	A	166	1674	1	0	Α	0	0	A	149	1405	0	0	Α	0	0	A	179	1717	1	0	Α
Street / Bridge Street		Right	1	0	Α	1424					0	0	Α	1184					1	0	A	1451				
Sueet	West	Left	19	9	В	207	207	19	9	В	14	5	Α	165	165	14	5	Α	17	8	В	207	207	17	8	В
		Left	14	31	A	606					13	17	A	488					17	19	В	615				_
	South	Thru	87	31	F	181	787	30	31	С	54	17	D	144	632	22	17	В	50	19	D	184	799	25	19	В
		Left	153	197	F	12					65	72	E	10					66	74	E	12				
	East	Thru	130	197		1071	1083	130	197	F	61	72	E	920	930	61	72	E	47	74	D	1100	1112	47	74	D
Epping Rd & Blaxland Rd &		Left	69	14	E	39					59	11	Е	32					47	12	D	39				
Langston PI	North	Thru	58	14	E	226	265	60	14	E	55	11	D	171	203	55	11	D	48	12	D	226	265	48	12	D
		Left	24	309	В	221					18	96	В	213					22	63	В	273				
	West	Thru	32 4	309	С	1582	2307	38	309	С	25	96	В	1438	2084	28	96	В	13	141	А	1835	2653	30	133	С
		Right	60	309	Е	504					42	96	С	433					91	141	F	545				
		Left	53	12	D	26					64	9	Е	22					53	12	D	26				
	South	Thru	63	12	Е	85	185	57	12	E	57	9	D	64	144	58	9	E	63	12	Е	85	185	57	12	E
		Right	52	12	D	74					57	9	D	58					52	12	D	74				
		Left .	11	7	Α	14					12	6	Α	13					11	7	Α	14				
	East	Thru	15	7	Α	676	690	14	7	Α	14	6	Α	543	556	14	6	Α	14	7	Α	676	690	14	7	Α
Epping Rd & Essex St		Left	131	56	F	2					121	23	F	1					126	46	F	2				
	North	Thru	86	56	F	81	506	85	56	F	52	23	D	68	423	56	23	D	71	46	F	83	520	73	46	F
	\$	Right	84	56	F	423					56	23	D	354					73	46	F	435				
		Left	29	20	С	62					19	9	В	50					29	60	С	62				
	West	Thru	11	20	A	1551	1613	12	20	Α	6	9	A	1426	1476	6	9	Α	17	60	В	1798	1860	17	60	В

B.2 PM Peak

Intersection Name	Approach Name	Movement Name				2023 Ba	ise Year							Stag	e1A	-	\ \					Stag	e4A			
			Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS
		Left	54	17	D	123					58	18	E	123		S			62	19	E	123				
	South	Thru	45	17	D	174	356	50	17	D	43	18	D	174	356	51	18	D	47	19	D	174	356	55	19	D
		Right	54	17 13	D A	59 109					59 2	18 16	E	59 109					63 1	19 15	E	59 109				
	East	Thru	8	16	A	1068	1184	8	16	Α	8	19	A A	1089	1206	8	19	Α	9	16	A A	109	1186	8	16	Α
Carlingford Rd & Ray Rd &	Lust	Right	19	15	В	7	1104	Ü	10		22	17	В	8		Ü	2.5		12	15	A	8	1100	Ü	10	
Rawson St		Left	41	10	С	85					41	12	С	85					41	10	С	85				
	North	Thru	37	10	С	132	234	40	10	С	43	12	D	131	233	44	12	D	36	10	С	131	233	40	10	С
		Right	56	10	D	17					68	12	E	17					60	10	E	17				
		Left	53	40	D	28					56	41	D	29					63	48	E	28				
	West	Thru	50	40	D	922	952	50	40	D	49	41	D	922	953	50	41	D	56	48	D	924	954	56	48	D
		Right	62	40	E	2					76	41	T	2					68	48	E	2				
	South	Left	31	12	С	880 1288	2168	34	59	С	26 32 4	102	С	899	2198	30	64	С	25 30	12 86	С	880	2171	28	56	В
		Thru	36 17	91 10	В	1288 889					16	102	В	1299 890					17	11	B	1291 890				
Beecroft Rd & Carlingford Rd	North	Right	75	26	E	305	1194	32	14	С	68	25	F	305	1195	29	14	С	66	24	E	305	1195	29	14	С
		Left	11	2	Α	74					13	2	A	74					12	3	A	74				
	West	Right	14	21	Α	987	1061	14	19	Α	14	20	Α	991	1065	14	19	Α	14	21	Α	992	1066	14	19	Α
		Left	11	4	Α	29				15.	21	4	В	29					18	4	В	29				
	South	Thru	17	4	В	91	196	16	4	В	17	4	В	91	196	19	4	В	17	4	В	91	196	19	4	В
		Right	18	4	В	76				$\mathcal{N}_{\mathcal{L}}$	22	4	В	76					23	4	В	76				
		Left	7	2	Α	34				$\boldsymbol{\mathcal{O}}$	6	2	Α	35					8	2	Α	34				
	East	Thru	8	2	Α	229	369	8		A	9	2	Α	234	378	9	2	Α	9	2	Α	231	371	9	2	Α
Bridge Street / Rawson		Right	10	2	Α	106					10	2	Α	109					10	2	Α	106				
Street		Left	45	4	D	9		10			46	5	D	9	228				40	3	С	9				
	North	Thru	12	4	Α	111	230	30 13	4	Α	15	5	Α	109	228	16	5	В	10	3	Α	110	229	12	3	Α
		Right	12	4	A	110					15	5	В	110					12	3	A	110				
		Left 	8	2	A	118		9			8	2	A	118	174				8	2	A	118				
	West	Thru Right	10 9	2	A A	44 12	174	9	2	А	13 10	2	A A	44 12	1/4	9	2	Α	11 9	2	A A	44 12	174	9	2	Α
	South	Left	11	0	A	17	17	11	0	A	15	0	A	17	17	15	0	A	7	0	A	17	17	7	0	A
Beecroft	Jouth	Left	0	0	A	115	1	- 11	0	Α	1	1	A	114	1/	15	Ů	^	1	1	A	113	17	,	Ů	
Road / High	East	Thru	2	0	A	354	2500	1	0	Α	1	1	A	359	2538	1	1	Α	1	0	A	355	2499	1	0	Α
Street / Bridge Street		Right	1	0	Α _	2031					1	1	A	2065					1	0	A	2031				
Silver	West	Left	26	7	В	128	128	26	7	В	55	11	D	127	127	55	11	D	41	12	С	130	130	41	12	С
	Counth	Left	45	42	D	837	077	47	42	D	46	43	D	840	000	47	42	D	56	53	D	845	988	rc	53	D
	South	Thru	58	42	E	140	977	47	42	D	52	43	D	140	980	47	43	U	61	53	E	143	300	56	23	D
	East	Left	38	98	C	29	1696	48	98	D	72	136	F	31	1729	57	140	Е	69	144	E	28	1683	58	144	E
Epping Rd & Blaxland Rd &		Thru	49	98	D	1667	1030		30	-	57	140	E	1698	2,23	3,	140	-	58	144	E	1655	1003	30	244	-
Langston PI	North	Left	70	13	F	23	242	54	13	D	71	27	F	23	242	69	27	Ε	68	13	Е	23	242	54	13	D
		Thru	52	13	D	219					69	27	Е	219					52	13	D	219				
		Left	9	10	Α	479					6	5	Α	478					6	6	Α	480				
	West	Thru	357	218	A	1130	1867	57	165	D	364	233	A	1131	1871	55	175	D	4 388	283	A	1130	1871	58	212	E
		Right		218		258					364 48	233	- 1	262					388 48	283	- 1	261				
	South	Left Thru	48 53	15 15	D D	46 188	293	53	15	D	48 53	15 15	D D	46 188	293	53	15	D	48 53	15 15	D D	46 188	293	53	15	D
	300111	Right	56	15	D	59	293	33	15	- U	56	15	D	59	233	33	13	-	56	15	D	59	253	33	13	
		Left	12	7	A	7					11	18	A	7					11	18	A	7				
	East	Thru	21	19	В	1195	1202	21	19	В	21	18	В	1199	1206	21	18	В	20	18	В	1196	1203	20	18	В
Epping Rd & Essex St		Left	171	101	F	4					145	96	F	4					146	100	F	4				
	North	Thru	122	101	F	121	562	120	101	F	121	96	F	121	562	116	96	F	124	100	F	119	560	120	100	F
		Right	119	101	F	437					115	96	F	437					119	100	F	437				
	V	Left	17	25	В	87	1124	20	25	c	11	15	А	89	1120	1.	15	Α	10	15	А	89	1125	1.	15	
	West	Thru	20	25	В	1034	1121	20	25		12	15	Α	1037	1126	11	15	А	12	15	Α	1036	1125	11	15	A

Intersection Name	Approach Name	Movement Name				Stag	e5B				Stage58(80%Demand)									Stage6							
			Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	Avg. Delay (S) Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue	Approach LoS	Avg. Delay (S)	Avg. Queue (m)	LoS	Tot Vehicles	Approach Volume	Approach Delay (S)	Approach Avg. Queue (m)	Approach LoS	
		Left	58	19	Е	123			(111)		53	13	D	99			(111)		55	17	D	123			(111)		
	South	Thru	48	19	D	175	357	54	19	D	42	13	С	139	285	47	13	D	44	17	D	173	355	49	17	D	
		Right	65	19	E	59					48	13	D	47) '		54	17	D	59					
		Left	5	16	Α	109					2	4	Α	93		~			4	14	Α	109					
	East	Thru	9	18	Α	1070	1187	9	18	Α	6	5	Α	844	945	6	5	Α	8	16	Α	1078	1195	8	16	Α	
Carlingford Rd & Ray Rd &		Right	12	16	Α	8					37	6	С	8					6	14	Α	8					
Rawson St		Left	40	12	С	85					43	8	С	70					41	9	С	85					
	North	Thru	48	12	D	131	233	46	12	D	36	8	С	105	188	38	8	С	34	9	С	131	233	38	9	С	
		Right	61	12	E	17					32	8	С	13					51	9	D	17					
		Left	64	48	E	27					35	22	С	21					67	52	E	28					
	West	Thru	56	48	D	905	934	56	48	D	38	22	C	739	762	38	22	С	60	52	Е	919	949	60	52	E	
		Right	66	48	Е	2					35	22	C	2					92	52	F	2					
	South	Left	24	11	В	882	2172	29	54	С	18	10	В	694	1747	25	25	В	24	12	В	887	2178	27	47	В	
		Thru	33	84	С	1290					30	35	С	1053					29	72	С	1291					
Beecroft Rd & Carlingford	North	Thru	17	15	В	890	1195	30	17	С	16	11	В	701	947	27	13	В	16	11	В	890	1195	29	14	С	
Nu		Right	67	24	E	305					57	17	E	246					68	24	E	305					
	West	Left Right	13 14	1 21	A A	74 973	1047	14	19	Α	8 16	0 19	A	60 794	854	16	18	В	12 14	1 20	A A	74 986	1060	14	19	Α	
		Left	23	6	B	29					1b 7	19	A	794 22					44	12	D D	29					
	South	Thru	22	6	B D	91	198	27	6		10	1	A	70	153	9	1	А	40	12	С	91	196	49	12	D	
	Joutii	Right	33	6	С	78	130	21	Ü		10	1	A	61	133	,	-	<u> </u>	62	12	E	76	190	43	12		
		Left	14	4	A	34					6	1	A	28					13	5	A	34					
	East	Thru	10	4	A	233	374	11	4	X	7	1	A	187	301	7	1	Α	16	5	В	235	376	16	5	D.	
Bridge Street	Last	Right	13	4	A	107	3/4		" /	$\mathcal{Y}_{\mathcal{P}_{i}}$	7	1	A	86	301	,	-	n n	17	5		107	370	10	,		
/ Rawson		Left	92	33	F	107					34	2	C	9					58	20	F	9					
Street	North	Thru	82	33	-	115	236	74	. 33	F .	9	2	A	89	189	10	2	Α	60	20	E	110	229	51	20	D	
		Right	64	33	Е	111					7	2	A	91			_		42	20	C	110				-	
		Left	12	4	A	119		. 0			7	2	A	95					16	6	В	118					
	West	Thru	38	4	C	46	177	19	4	В	11	2	A	95 38	143	8	2	Α	43	6	С	44	174	24	6	В	
		Right	18	4	В	12					7	2	A	10					29	6	В	12				_	
	South	Left	17	0	В	17	17	17	0	В	17	0	В	14	14	17	0	В	25	0	В	17	17	25	0	В	
Beecroft		Left	0	1	Α	113					0	0	Α	98					1	1	Α	115					
Road / High	East	Thru	1	0	Α	357	2499	1	0	Α	0	0	Α	287	2017	0	0	Α	1	0	Α	358	2505	1	0	Α	
Street / Bridge Street		Right	1	0	Α	2029					0	0	Α	1632					1	0	Α	2032					
	West	Left	78	29	F	135	135	78	29	F	18	5	В	110	110	18	5	В	85	38	F	131	131	85	38	F	
	6. 11	Left	41	41	С	836	075				34	27	С	669	770	27	27		43	41	D	840	000	45		D	
	South	Thru	62	41	E	140	976	44	41	D	59	27	E	110	779	37	27	С	53	41	D	140	980	45	41	U	
	East	Left	62	116	E	28	1691	50	116	D	48	66	D	24	1368	35	66	С	56	97	D	29	1698	42	97	С	
		Thru	50	116	D	1663	1031	30	110	b	34	66	С	1344	1300	33	00	C	42	97	С	1669	1050	42	37	C	
Epping Rd & Blaxland Rd & Langston Pl	North	Left	82	14	· JE	23	242	62	14	F	60	11	E	19	198	59	11	F	69	13	E	23	242	54	13	D	
_	140/111	Thru	60	14	E	219	2-12			-	59	11	E	179	130	33		-	53	13	D	219	2-12	54	-13		
		Left	14	117	Α	474					10	49	Α	375					6	5	Α	476					
	West	Thru	32	117	С	1094	1836	34	117	С	19	49	В	888	1481	22	49	В	4	229	Α	1128	1867	55	172	D	
		Right	76	117	F	268					56	49	D	218					362	229	F	263					
		Left	48	15	D	46					55	11	D	32				_	48	15	D	46					
	South	Thru	53	15	D	188	293	53	15	D	50	11	D	150	230	51	11	D	53	15	D	188	293	53	15	D	
		Right	56	15	D	59					51	11	D	48					56	15	D	59					
	East	Left	11	18	A	7	1205	20	18	В	16	12	В	6	966	18	13	В	11	18	A	7	1204	20	18	В	
Epping Rd & Essex St		Thru	20	18	В	1198					18	13	В	960					20	18	В	1197					
	Nesth	Left	145 120	97 97	F	4 121	562	116	07		30 54	22	С	3 99	463	54	22	D	115 113	89 89	F	4	563	109	90		
	North	Thru		97		121 437	562	116	97		54 54	22 22	D D	99 361	463	54	22	U	113	89	F	122 437	563	109	89		
		Right Left	115 21	18		437 86					54 20	10	D	361 74					107	89 15	_	437 89					
	West	Thru	21 15	18	B	1020	1106	16	18	В	20 11	10	B A	74 818	892	12	10	Α	10	15	A A	1035	1124	12	15	Α	
		iiid	15	10		1020					- 11	10	М	010					12	10	м	1033					

C. Project Case Drawing

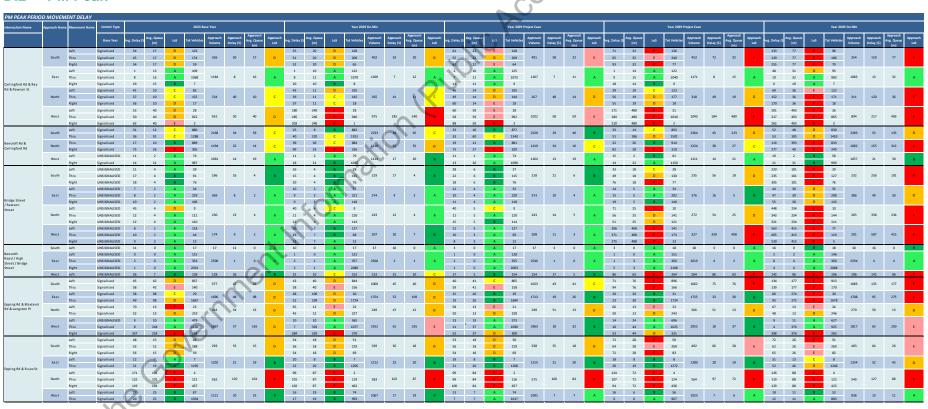


D. Intersection Performance - Future Year Scenarios

D.1 AM Peak



D.2 PM Peak



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